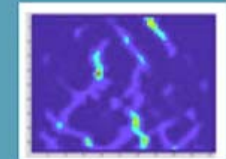
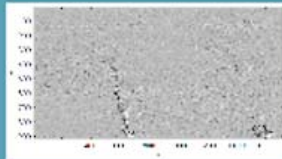




Workshop 4 , June 9

Recent progress in Digital Image Correlation: towards integrated identification?



Two civil and industrial applications of 2D DIC measurements combined with numerical simulations

Roberto Fedele and coworkers:

M. Scaioni, G. Rosati. M. Ferraris, V. Casalegno



Department of Civil and Environmental Engineering (DICA)

Politecnico di Milano, Milan, Italy

Items to be discussed

- ✓ Delamination tests on FRP-reinforced masonry pillar, optical monitoring by 2D DIC and FE predictions

coworkers: M. Scaioni, G. Rosati



- ✓ Shear tests on metal-ceramic assemblies, identification of cohesive parameters for innovative joints

coworkers: M. Ferraris, V. Casalegno



- ✓ Closing remarks and future prospects

Delamination experiments and FE modelling of FRP-reinforced masonry

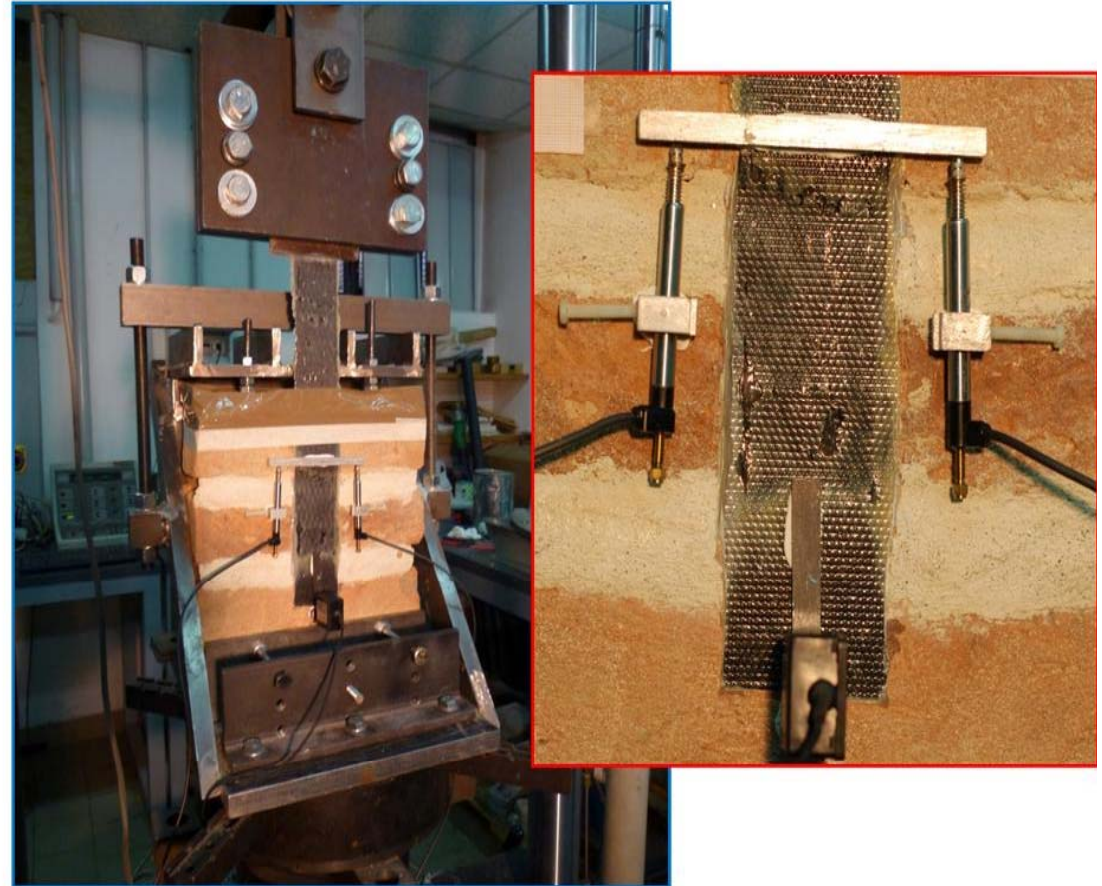
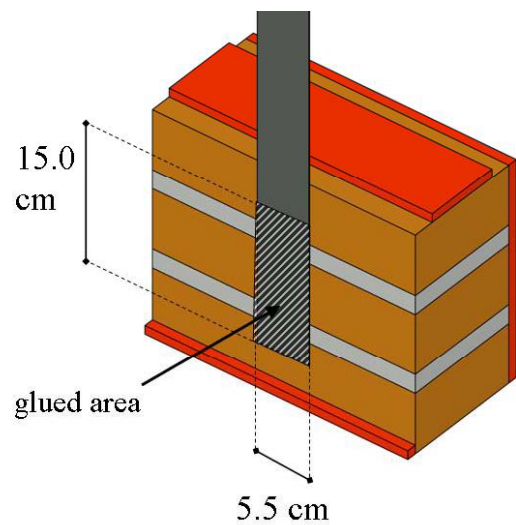
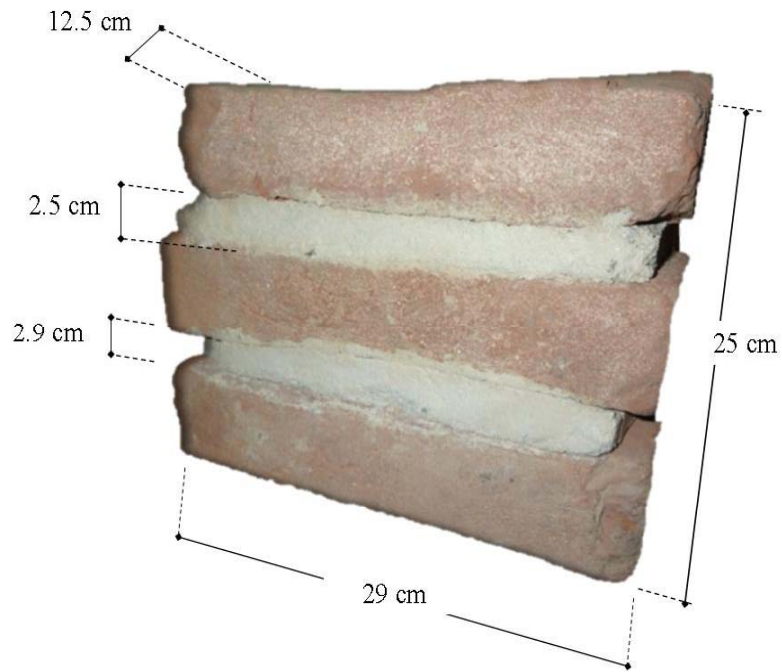
Roberto Fedele, M. Scaioni, G. Rosati

Ref: Fedele et alii, Cement & Concrete Composites, 45 (2014)
243–254.



Department of Civil and Environmental Engineering (DICA)
Politecnico di Milano, Milan, Italy

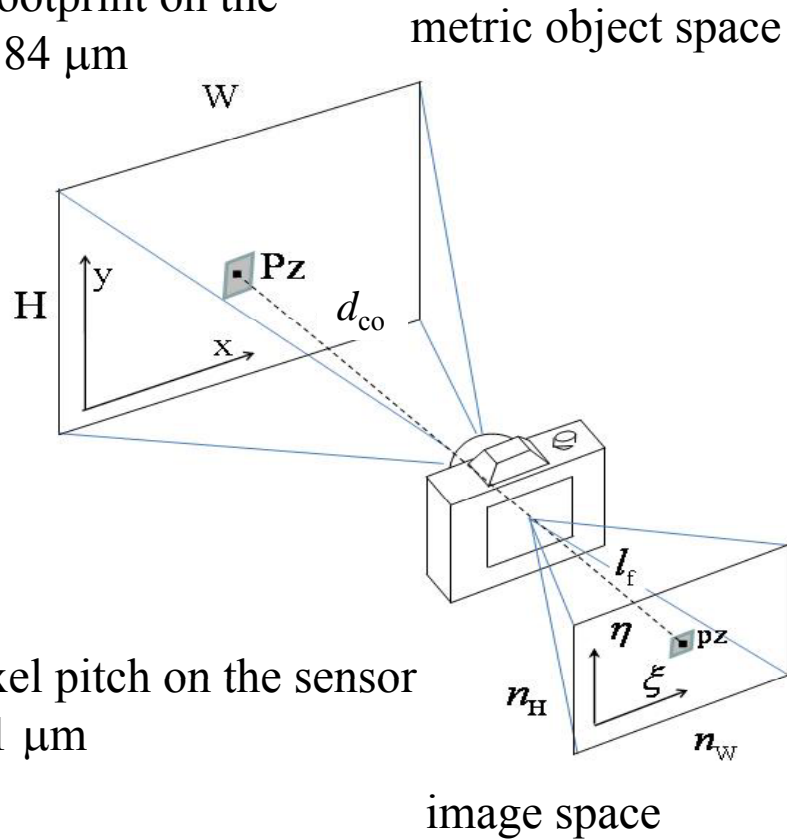
CFRP-reinforced pillar: Historical bricks (XVII century) and high strength mortar



single-lap shear test

Optical monitoring by 2D DIC

1 pixel footprint on the object $\cong 84 \mu\text{m}$



1 pixel pitch on the sensor
= $6.1 \mu\text{m}$

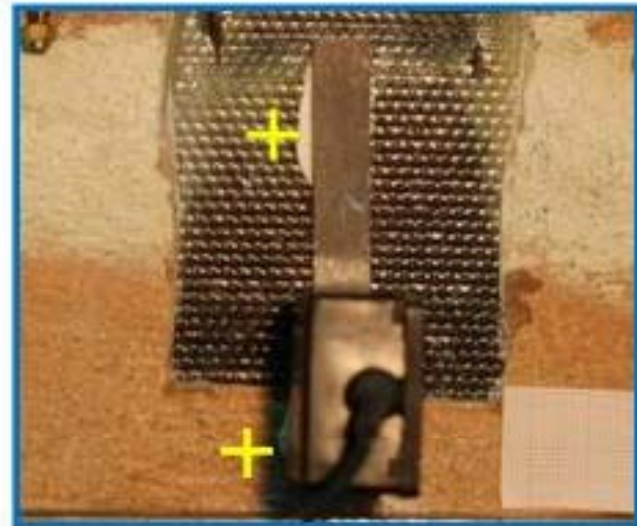
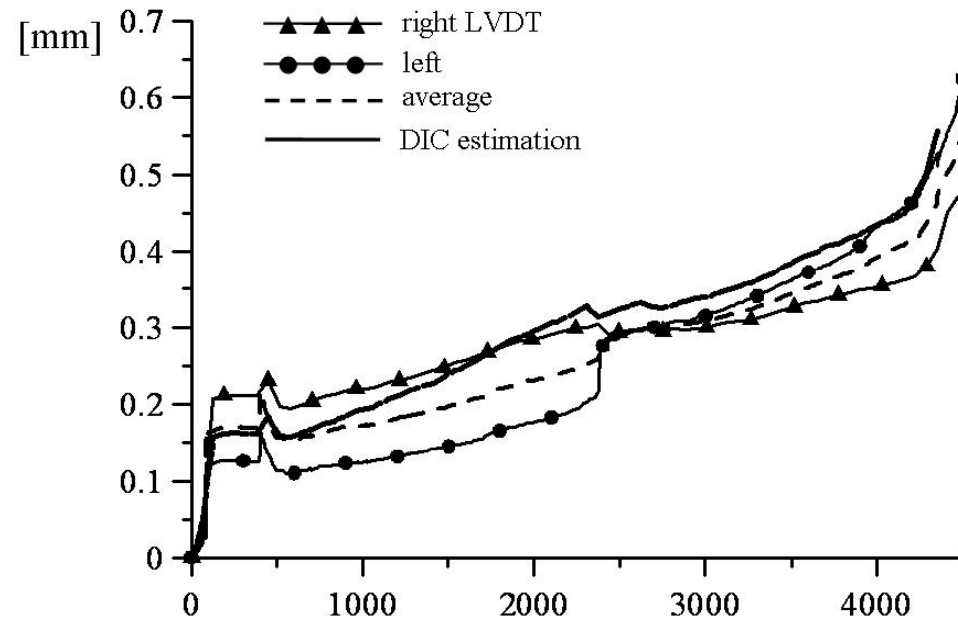


Benchmarking

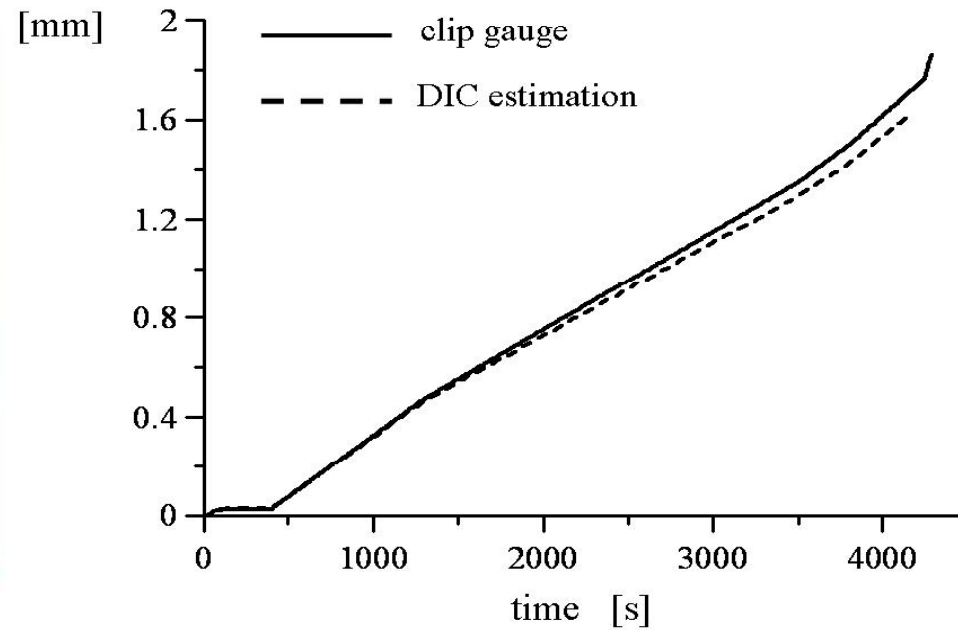
RMSE= $15.9 \mu\text{m}$ (0.19 Pz)

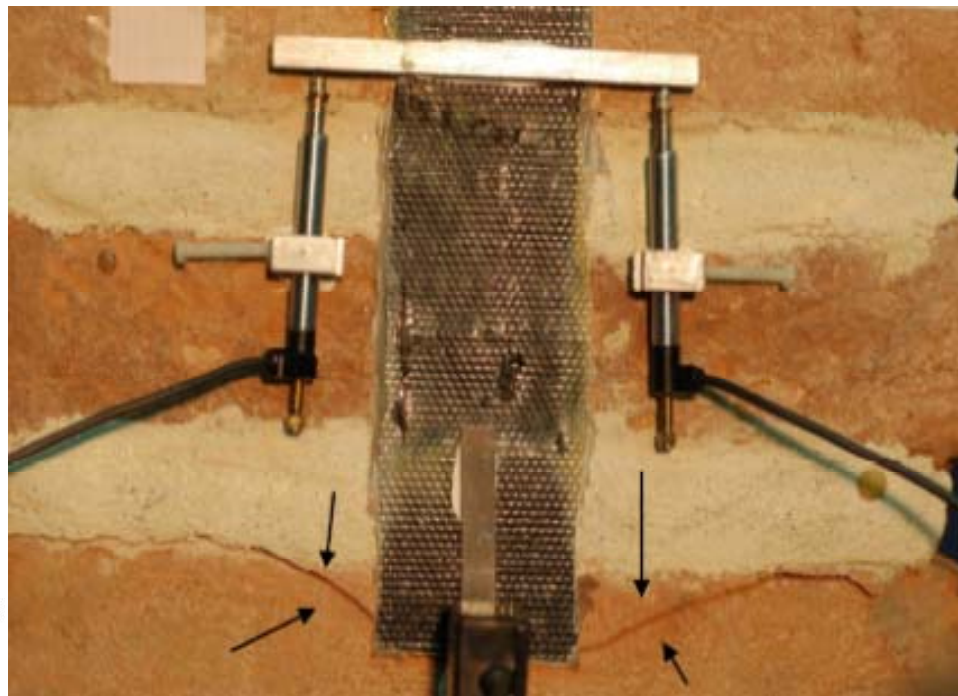
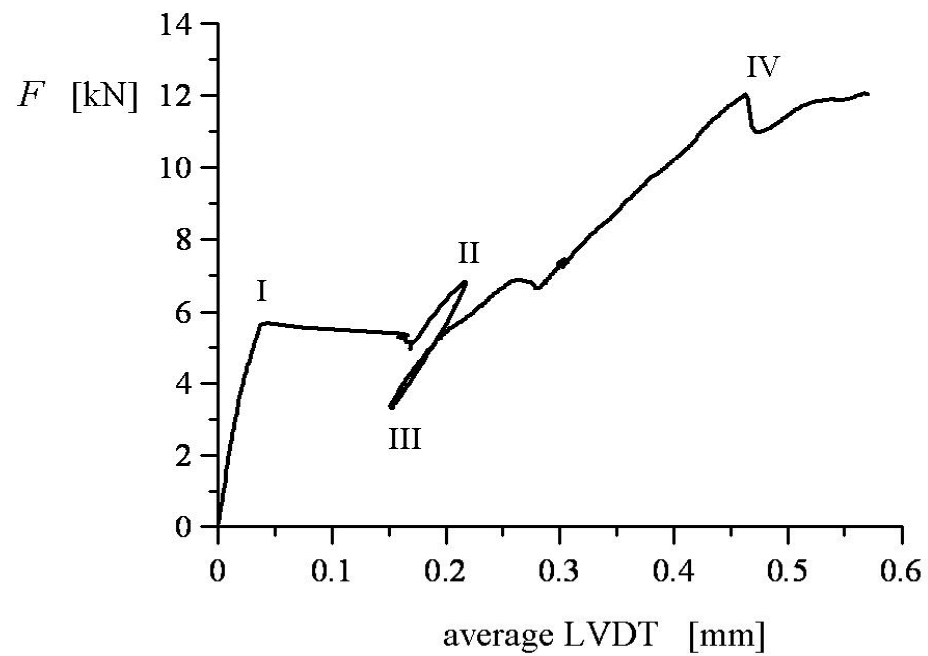
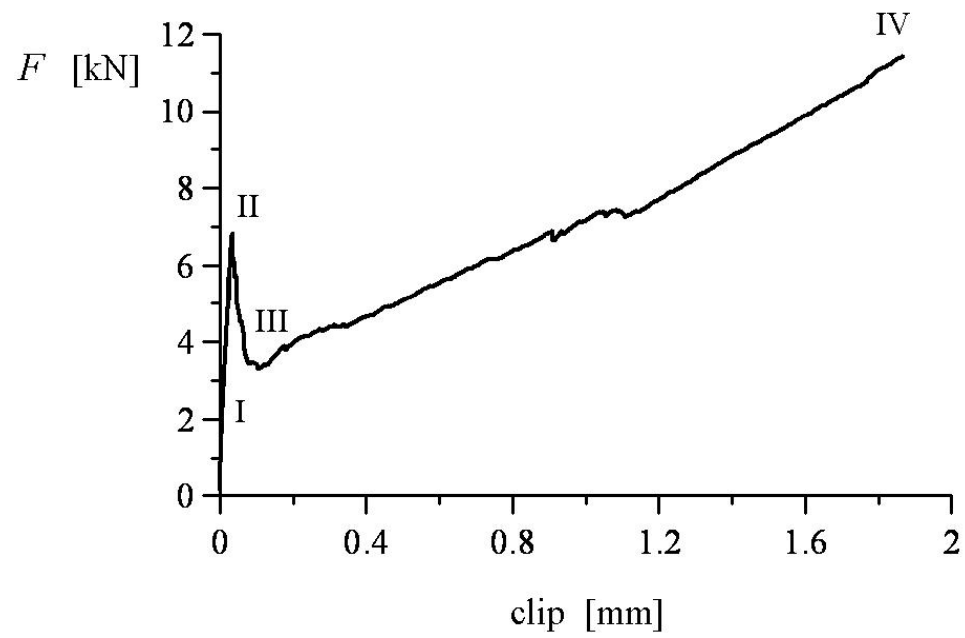
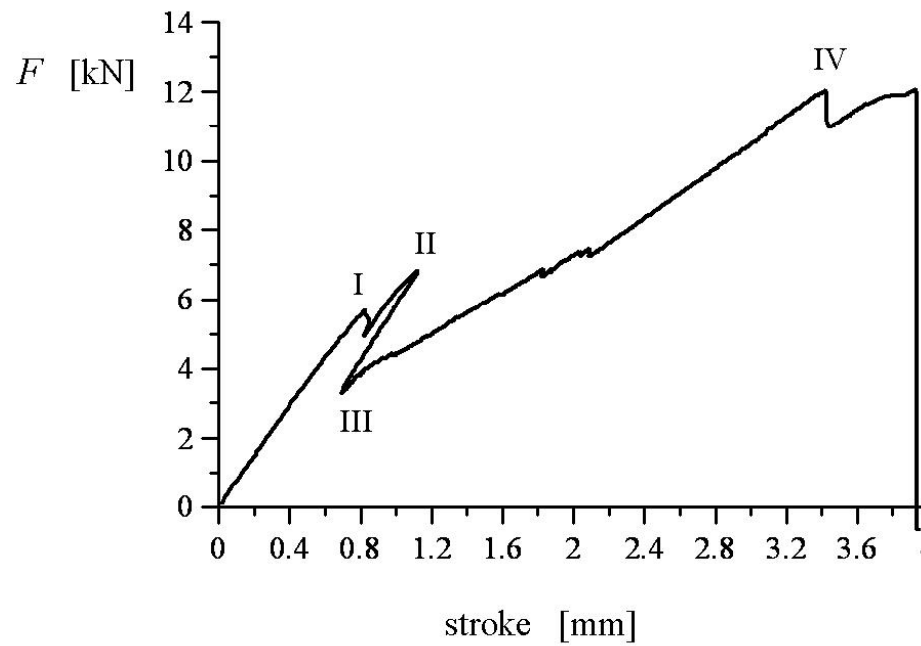


displacement

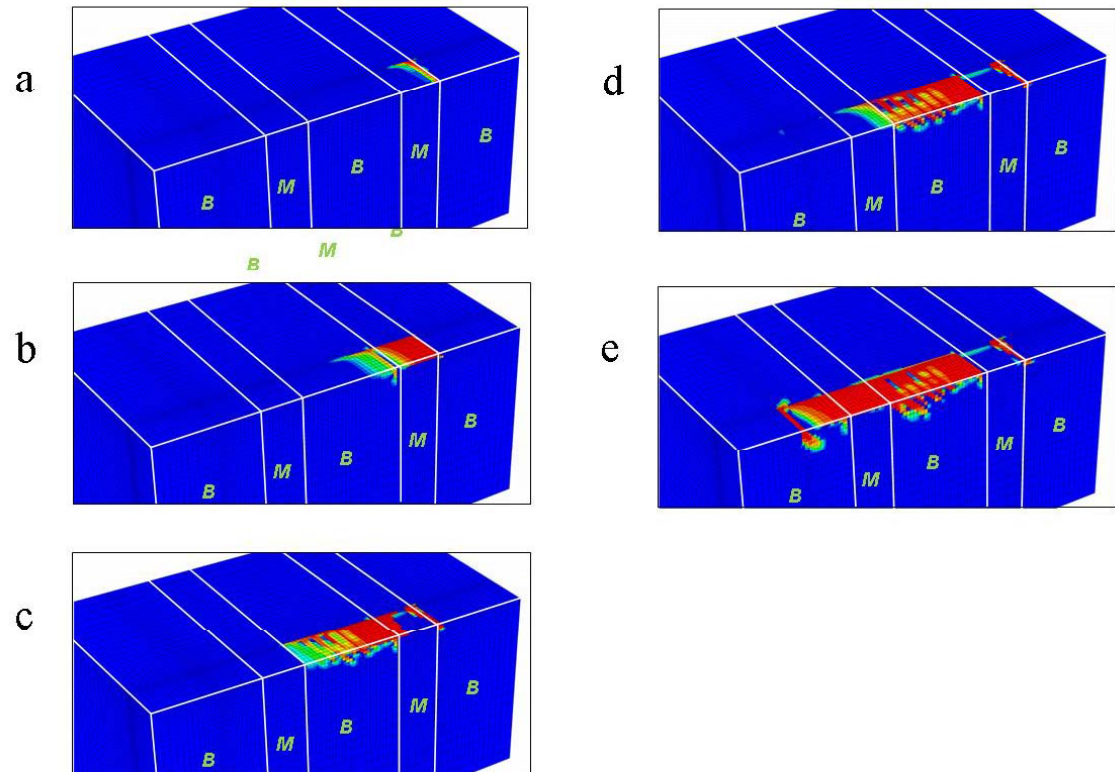
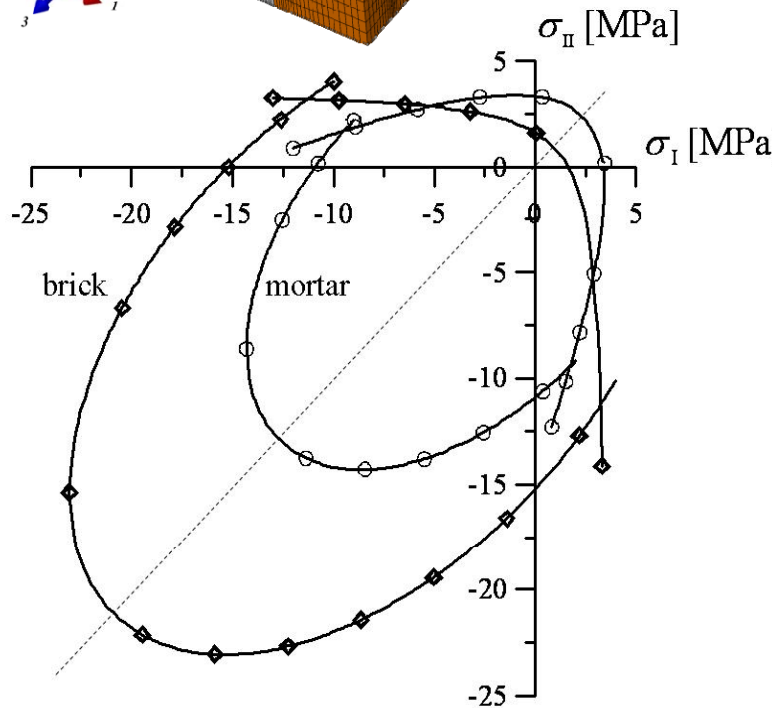
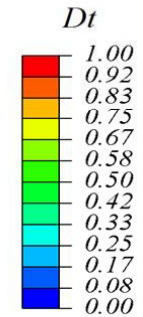
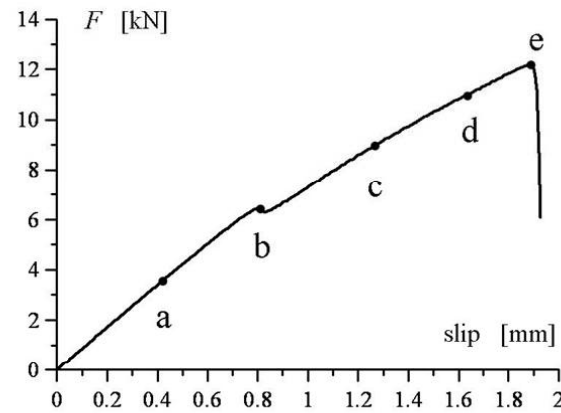
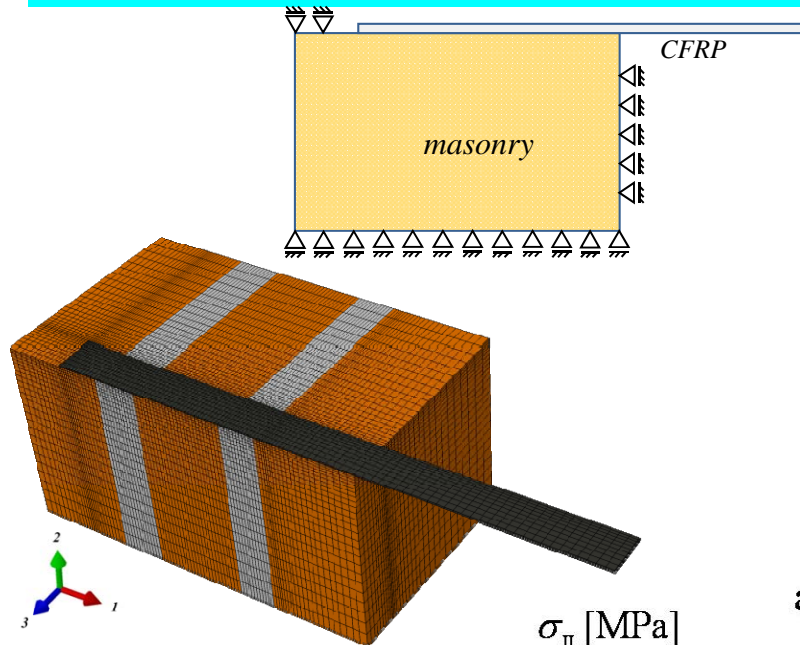


displacement





3D heterogeneous finite element modelling



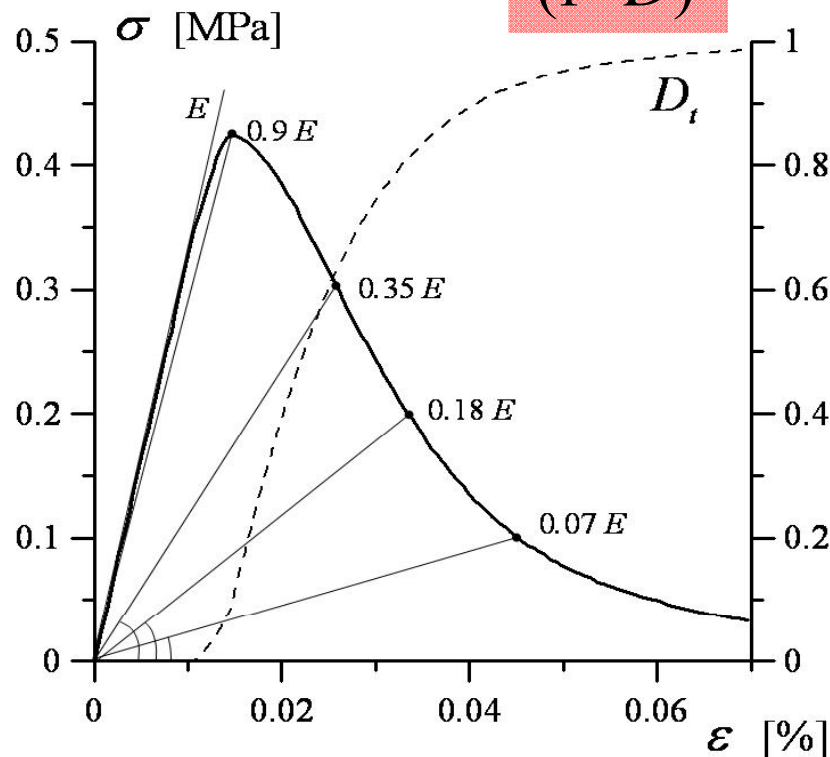
Elastic-damageable model (Comi-Perego 2001)

“bi-dissipative” model

Vumat (Abaqus[®] explicit)

state equations

$$\sigma_{ij} = \underbrace{(1 - D_t) \cdot (1 - D_c)}_{(1 - D)} \left[\left(K - \frac{2}{3} G \right) \varepsilon_v \delta_{ij} + 2G \varepsilon_{ij} \right]$$



loading-unloading conditions

$$f_t(\sigma_{ij}, D_t) \leq 0$$

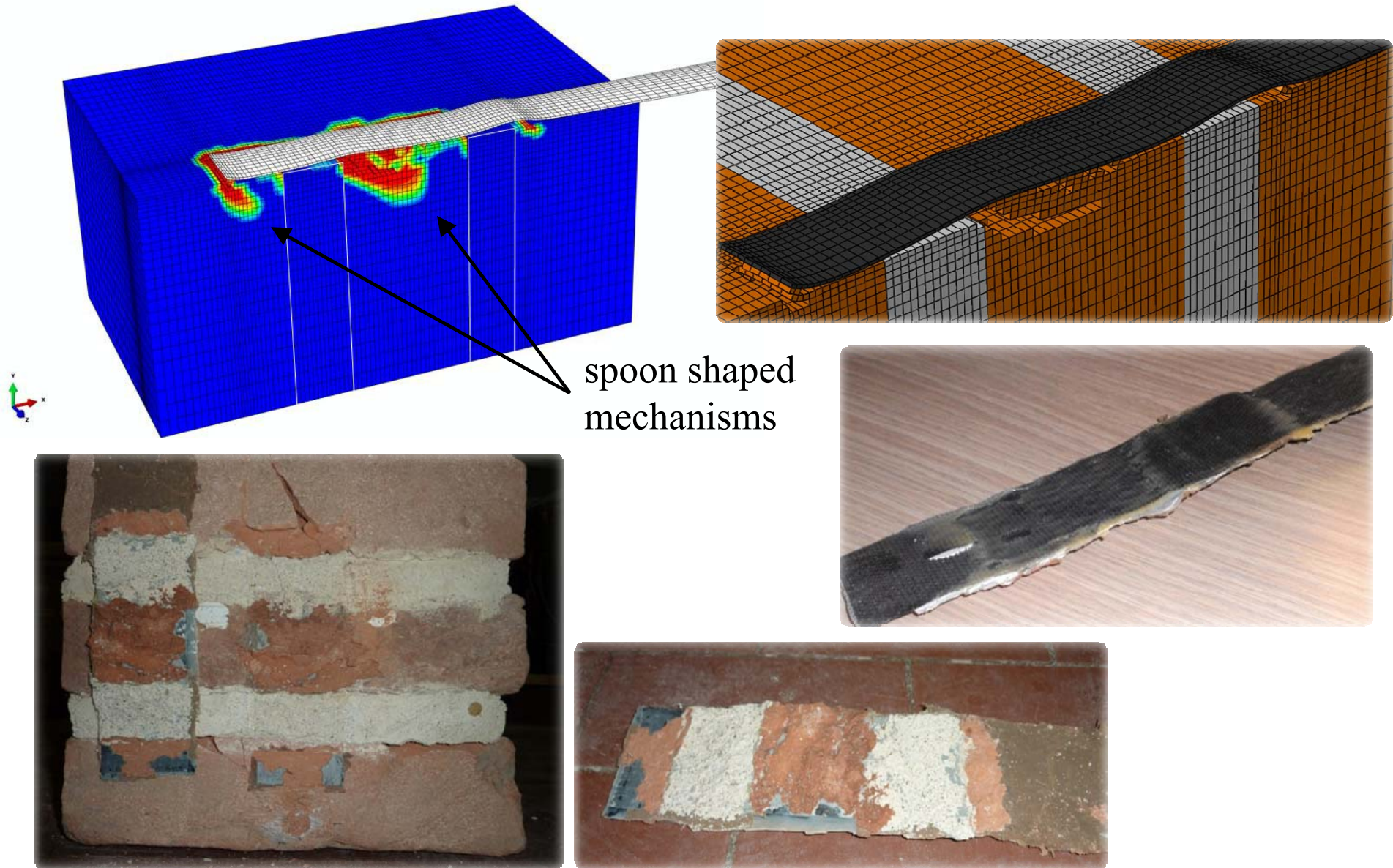
$$\dot{D}_t \geq 0 \quad f_t \dot{D}_t = 0$$

$$f_c(\sigma_{ij}, D_c) \leq 0$$

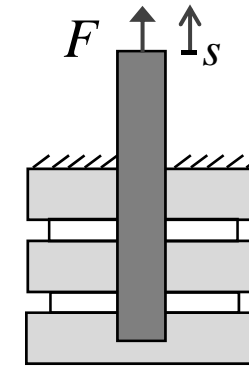
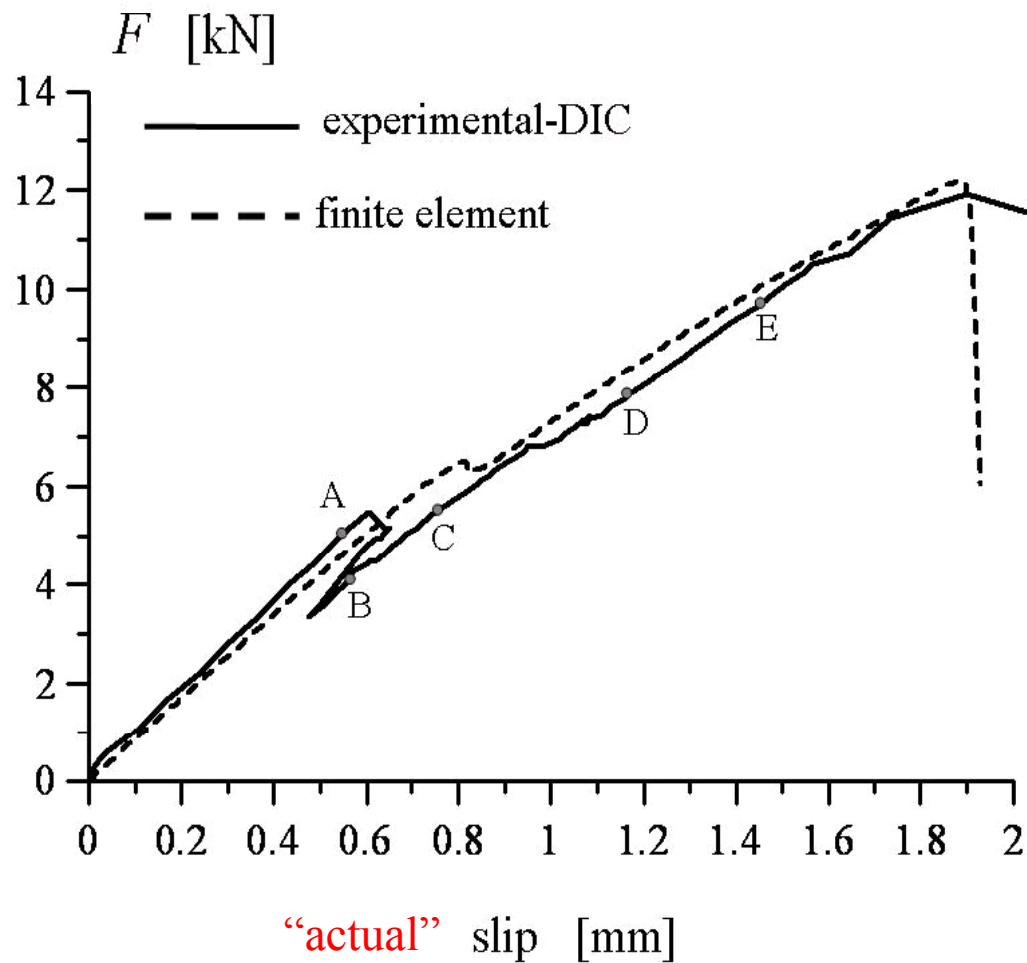
$$\dot{D}_c \geq 0 \quad f_c \dot{D}_c = 0$$

3D FE modelling with perfect adhesion

Effective elastic modulus of the CFRP reinforcement estimated by DIC

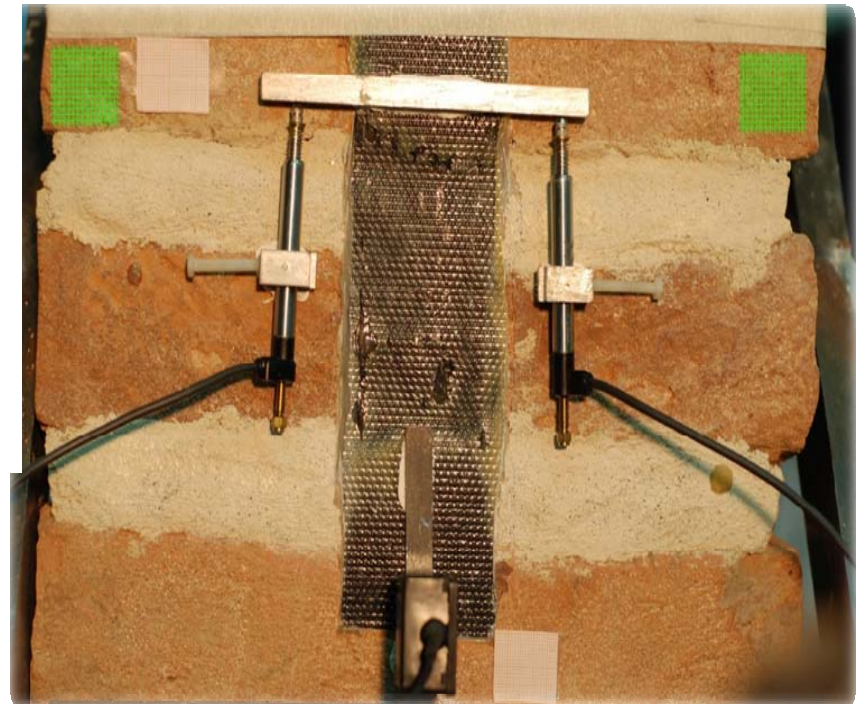


Overall delamination response

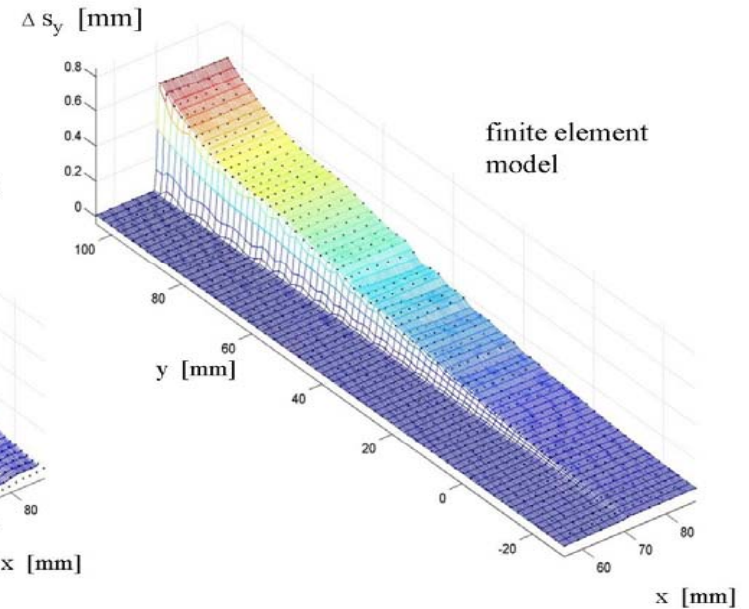
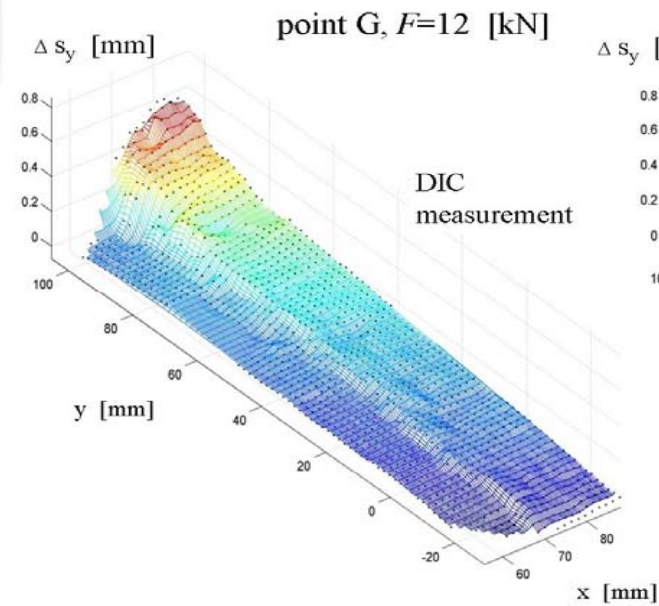
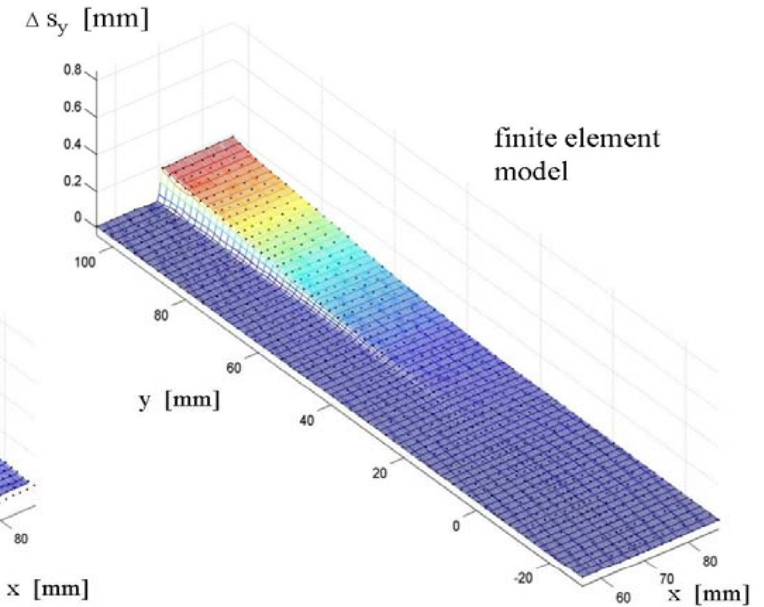
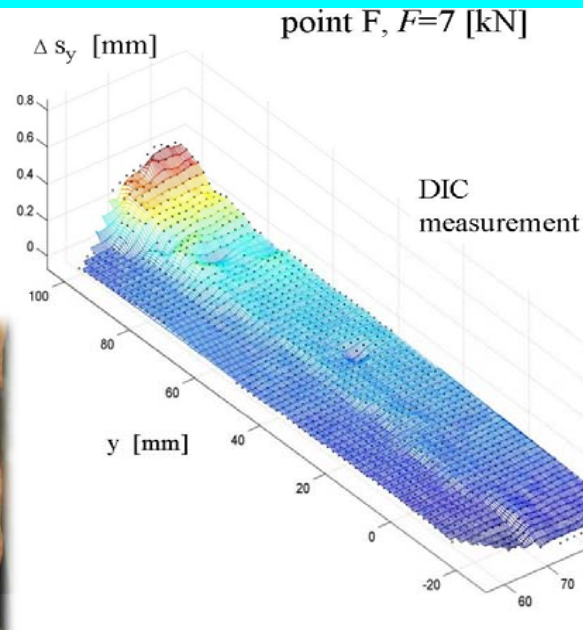
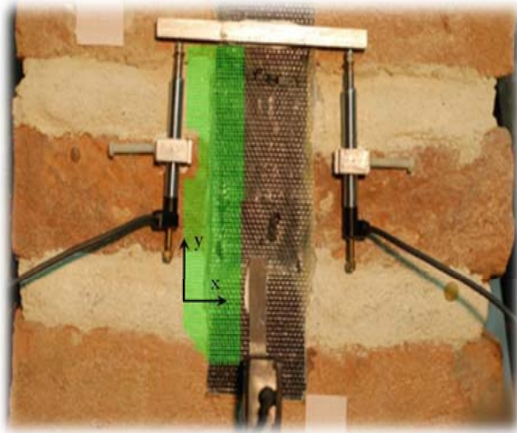


FE model with
"ideal" constraints

DIC-corrected boundary conditions



Local slip between FRP and masonry support



Characterization of innovative CFC/Cu joints by full-field measurements and finite elements

Roberto Fedele^{*}, Valentina Casalegno[#], Monica Ferraris[#]

Ref: Fedele et alii, Materials Science & Engineering A, 595 (2014)
306–317.

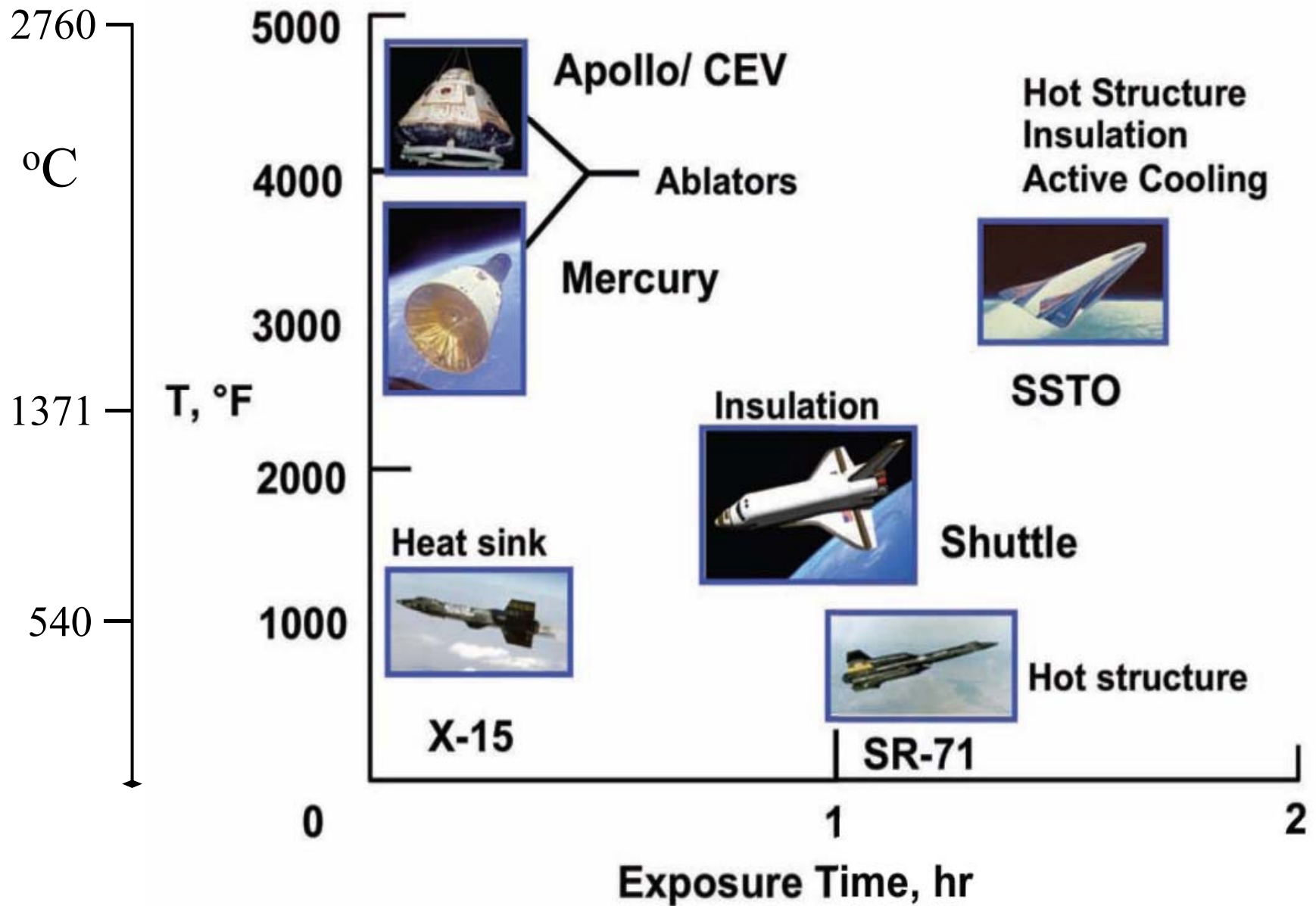


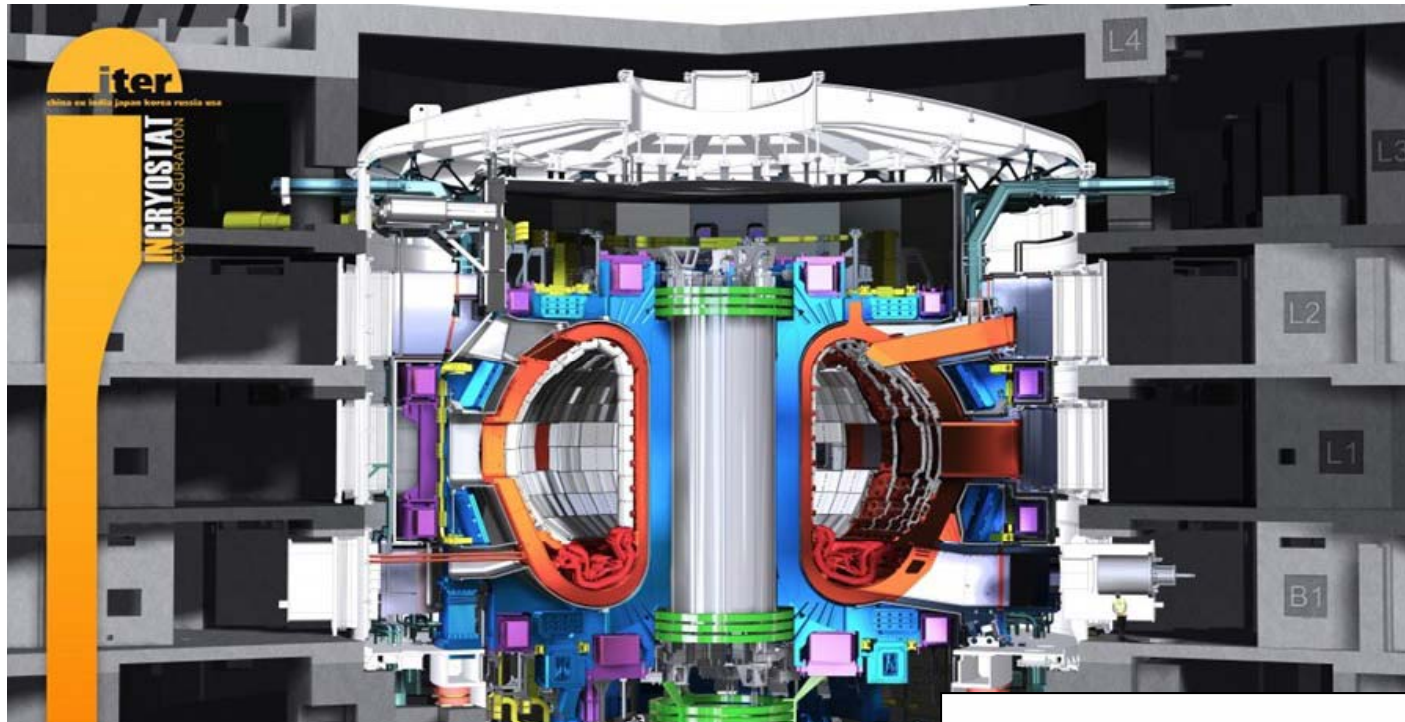
^{*}Dept. of Civil and Environmental
Engineering (DICA)
Politecnico di Milano, Milan, Italy



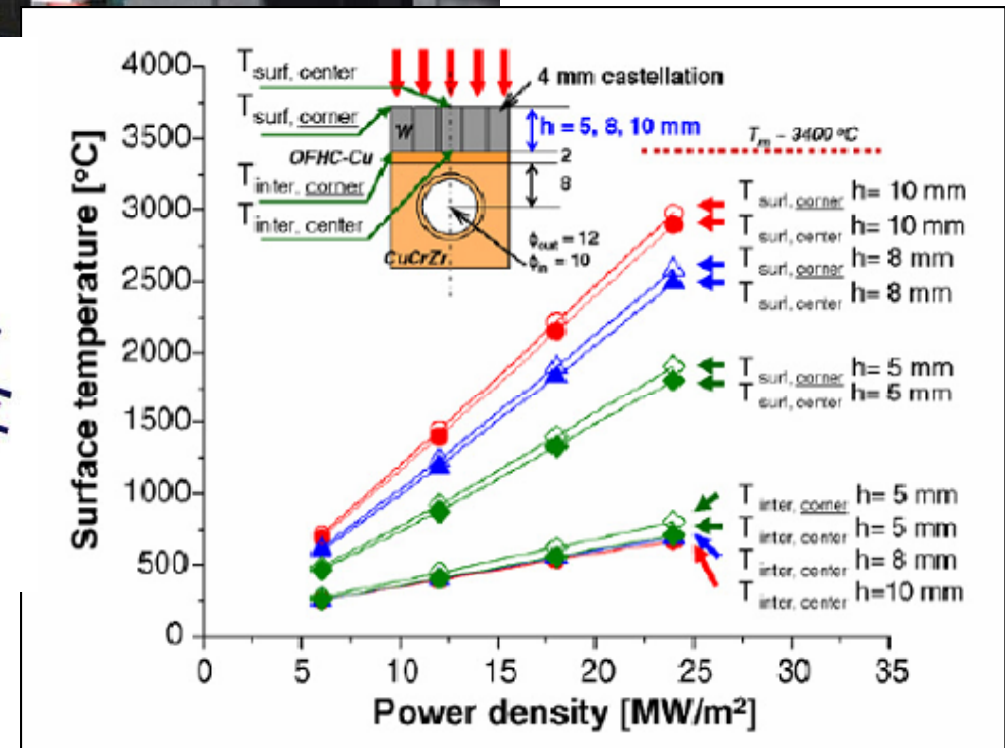
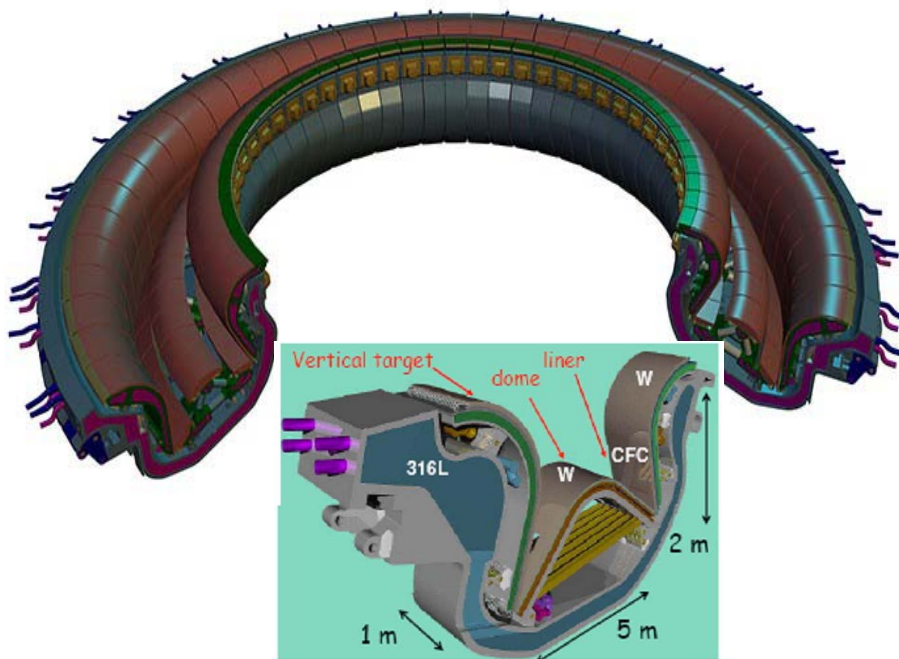
[#] Dept of Applied Science
and Technology (DISAT)
Politecnico di Torino, Turin, Italy

Composites for aggressive environments





ITER (International Thermonuclear Experimental Reactor)



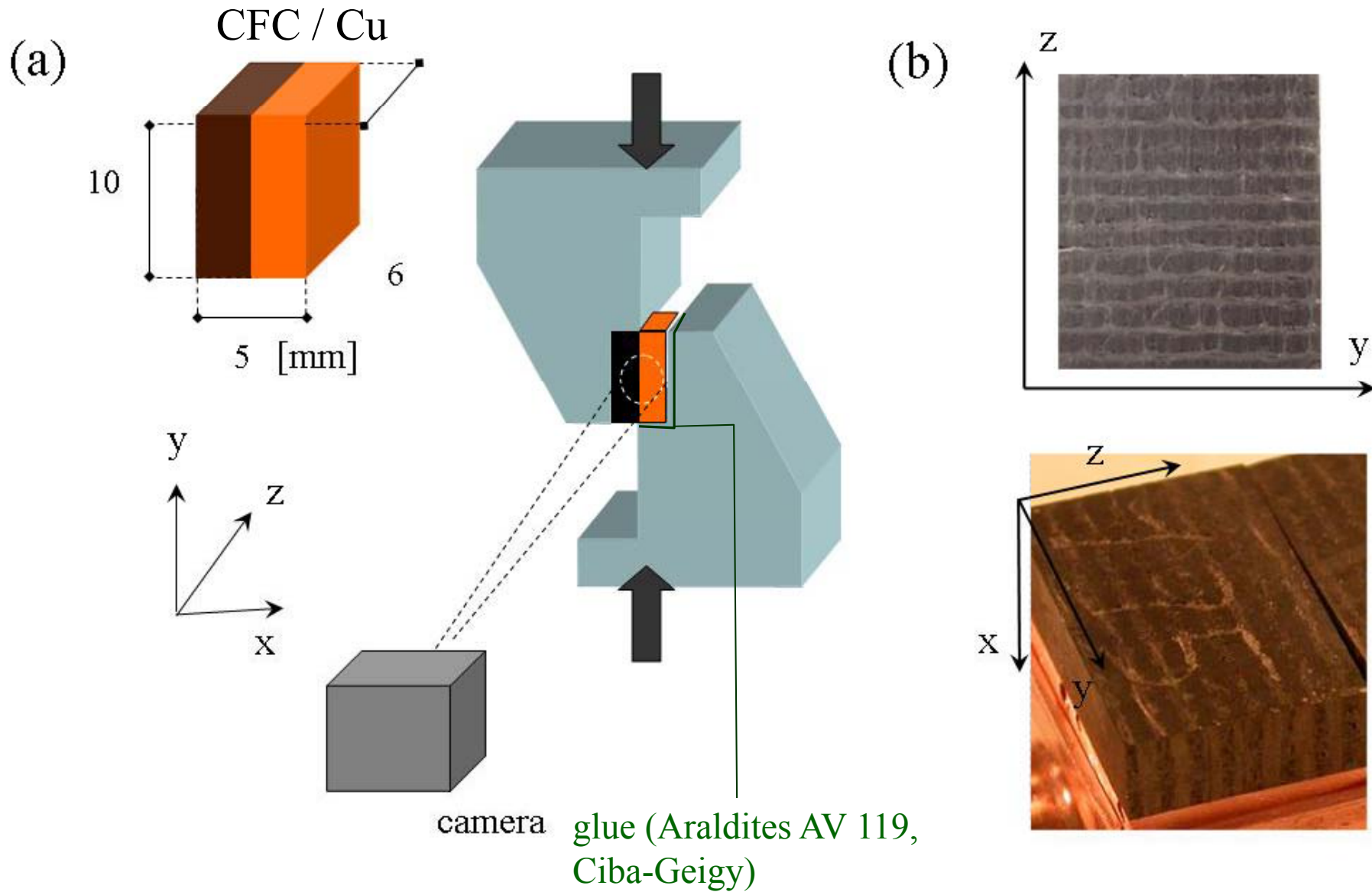


brake discs

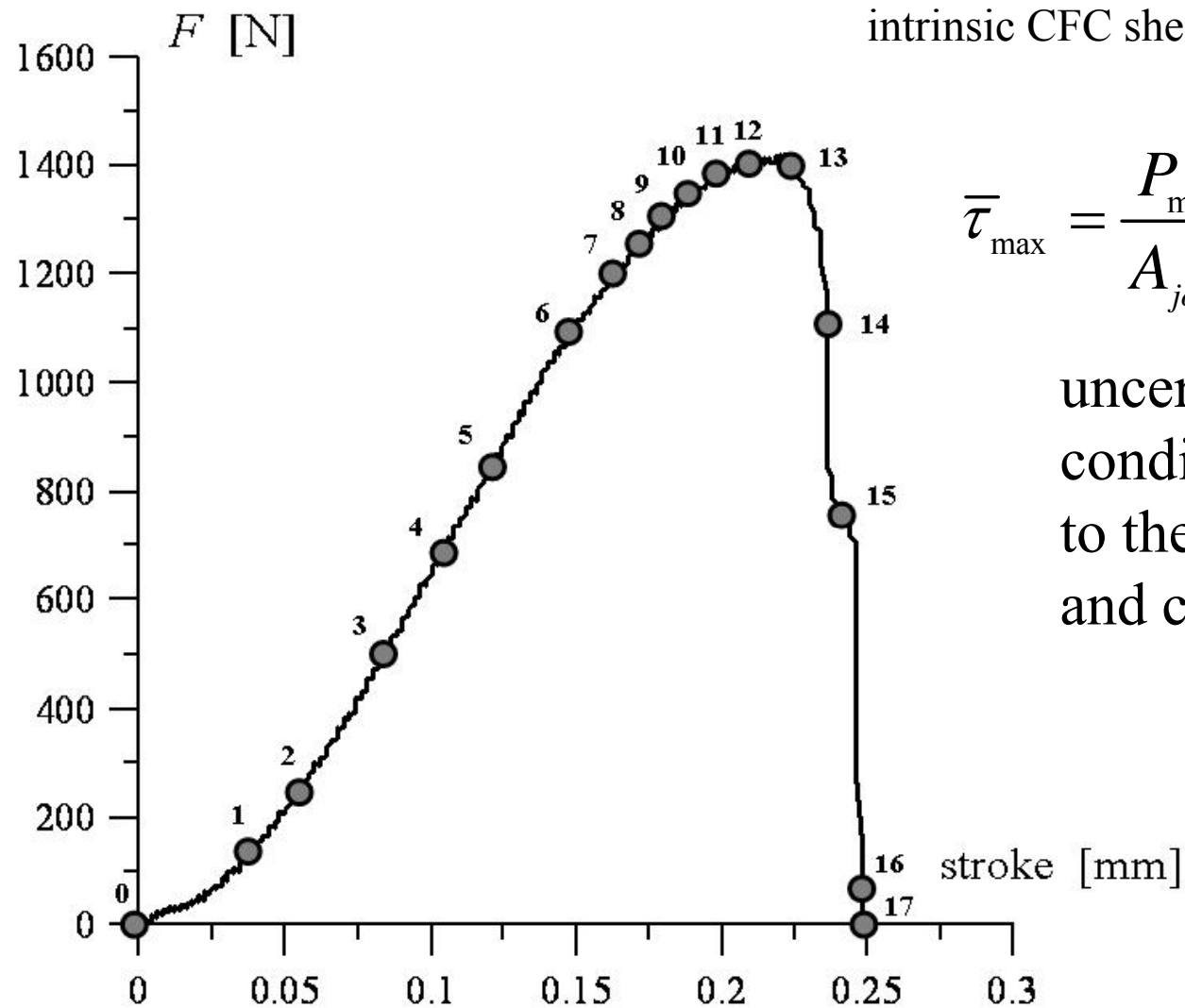
turbine engine blades



Single-lap shear tests on flat-tile joined samples



Macroscopic response



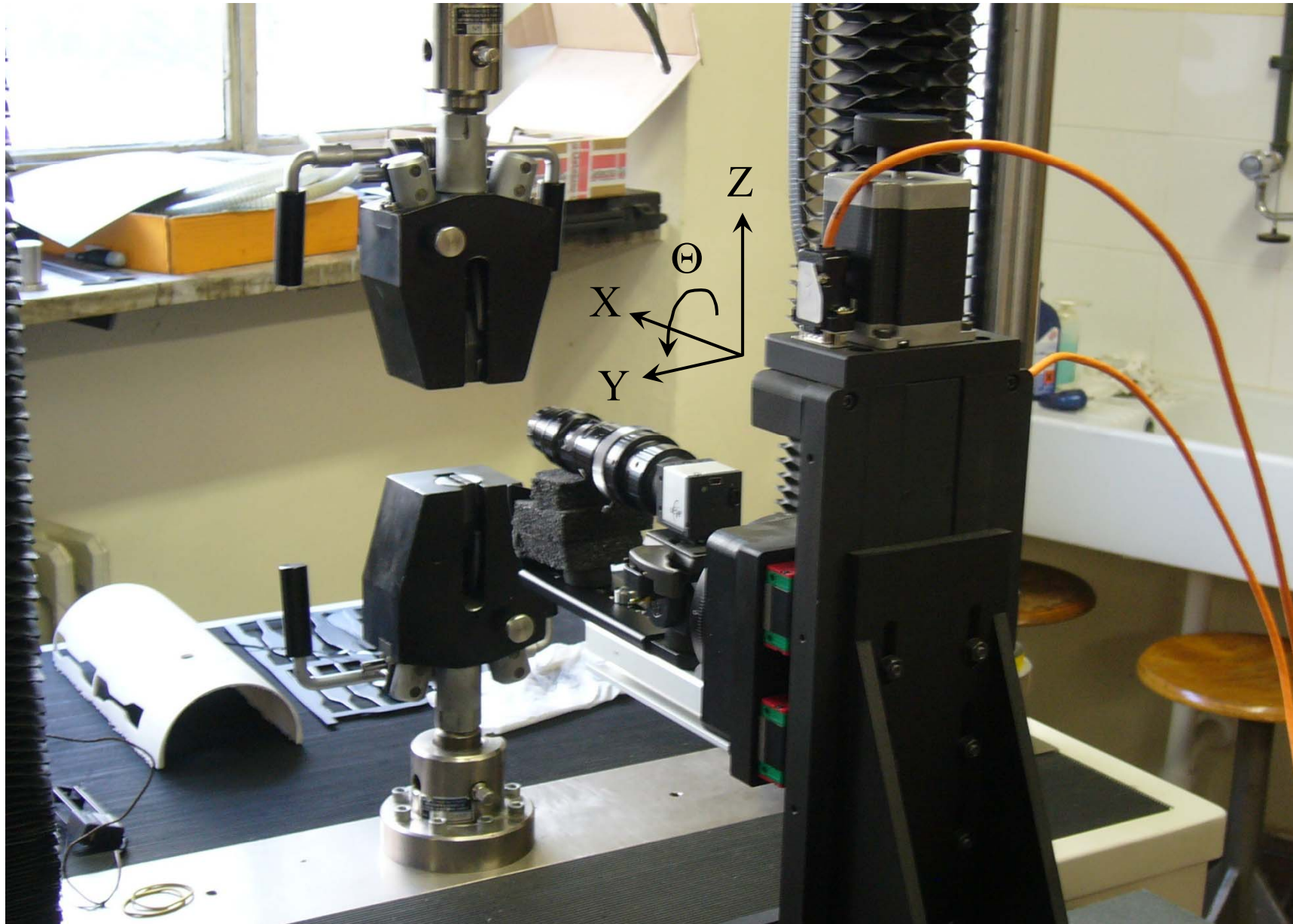
intrinsic CFC shear strength 15-20 MPa !

$$\bar{\tau}_{\max} = \frac{P_{\max}}{A_{\text{joint}}} \cong 23.6[\text{MPa}]$$

uncertain boundary conditions due to the glue layers and compliant grips!

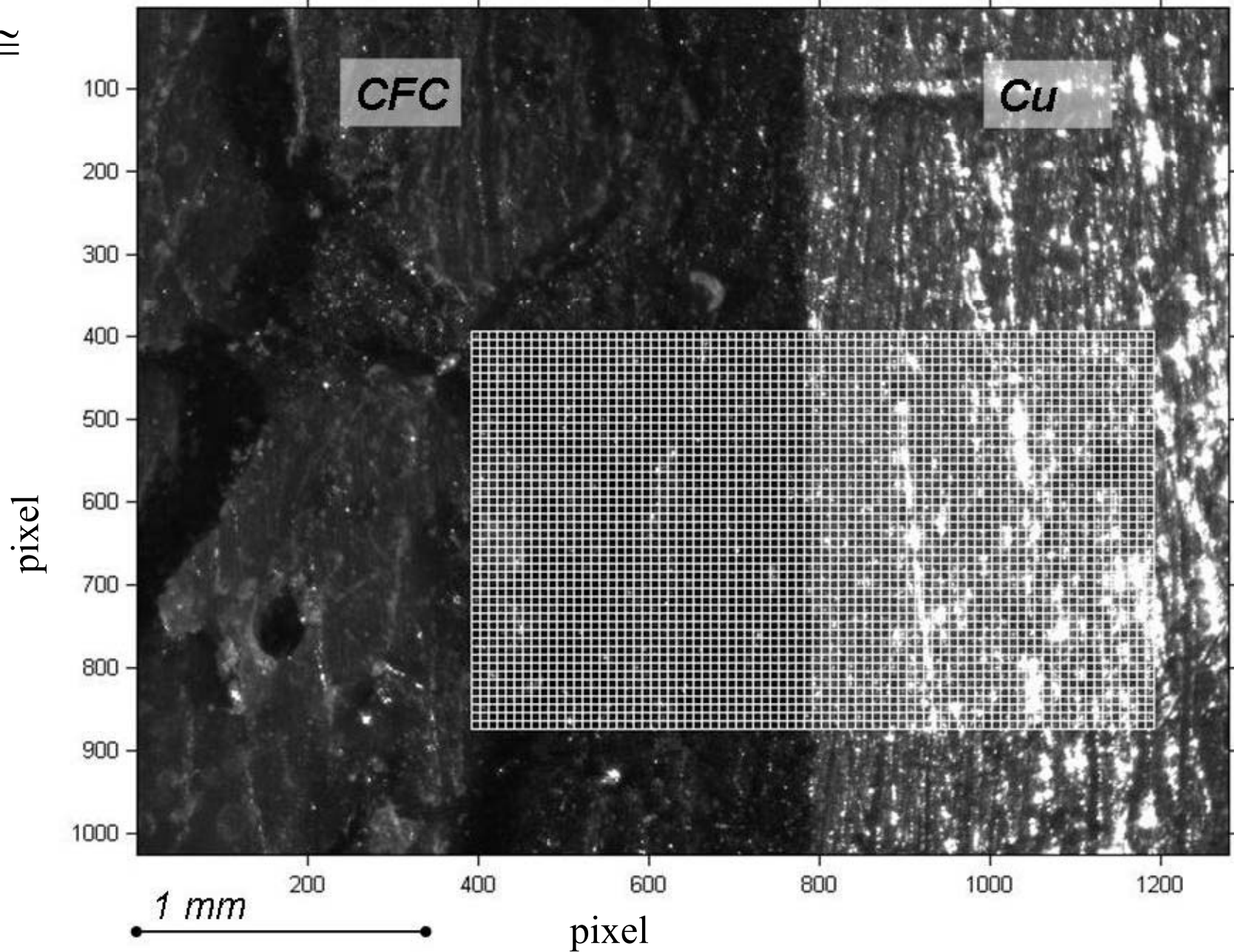
comparative assessment by different tests $\bar{\tau}_{\max} = 34 \pm 4 [\text{MPa}]$

4-dofs motion for zoomed camera for optical monitoring

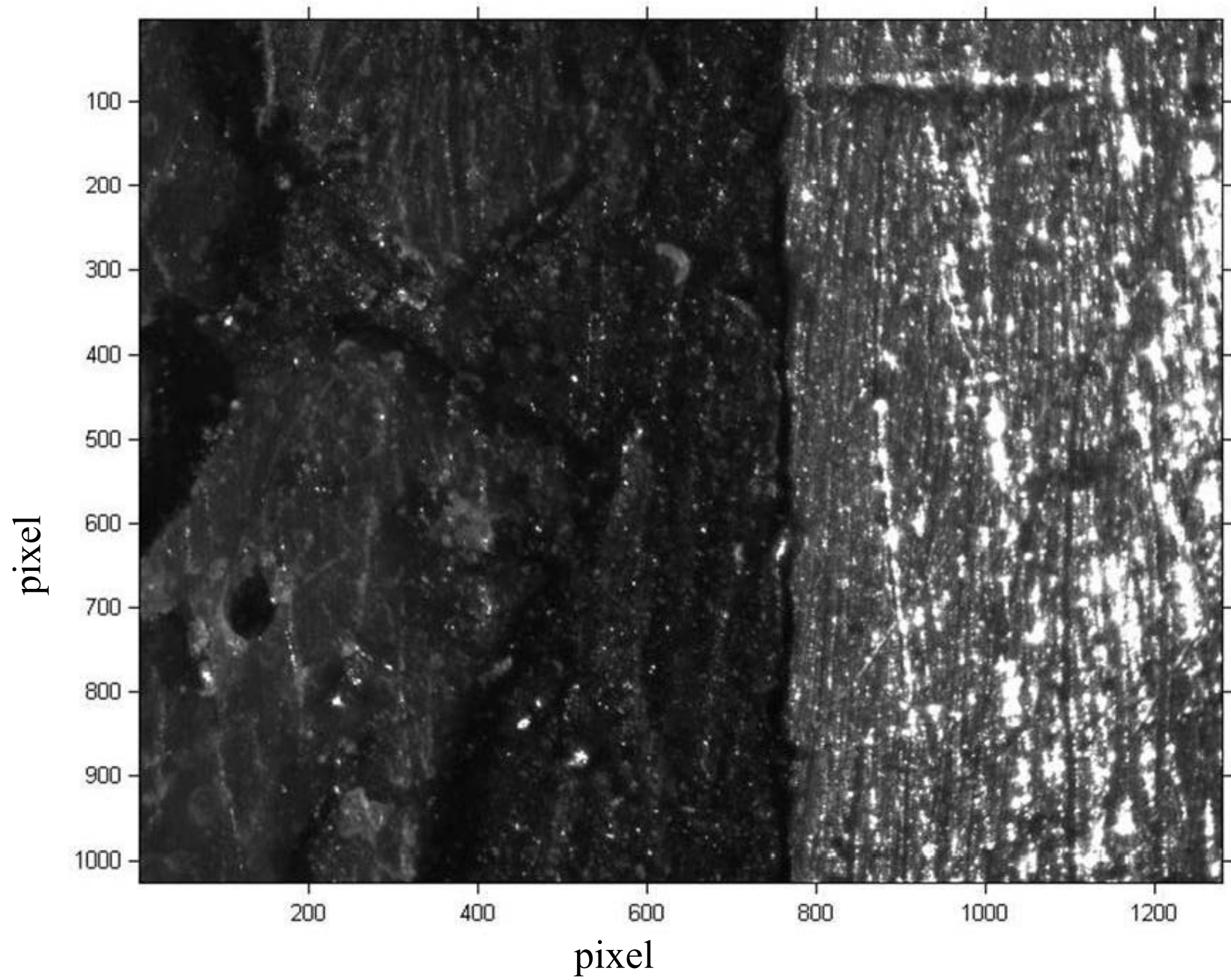


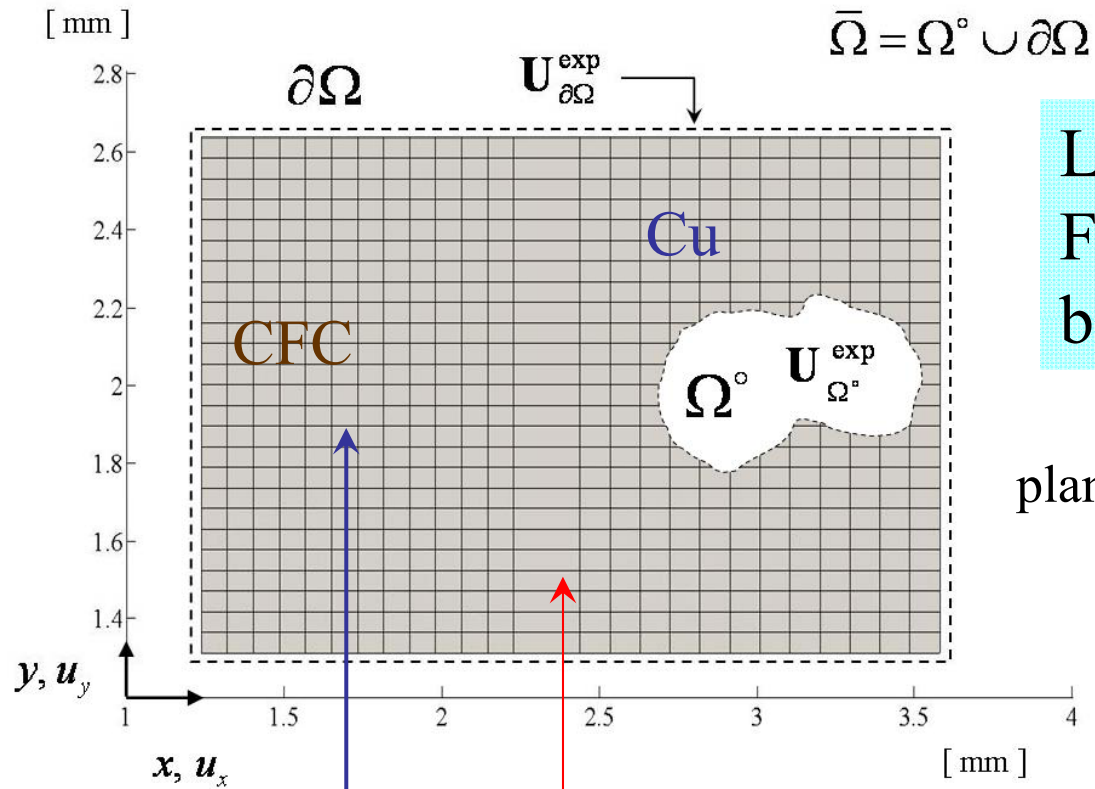
Finite element discretization of ROI

1 pixel \cong
3.0 μm



Joint collapse



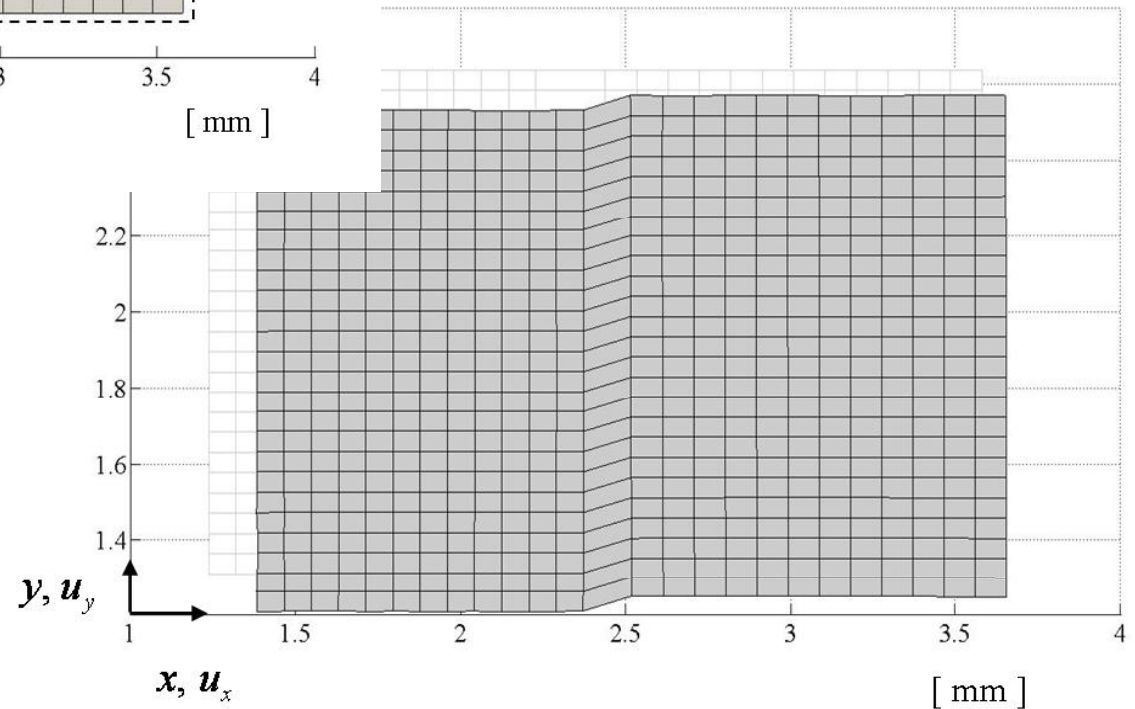


Local approach:
FE simulations driven by
boundary displacements

plane stress schematization

(anisotropic)
elastic-plastic
behavior

finite thickness joint

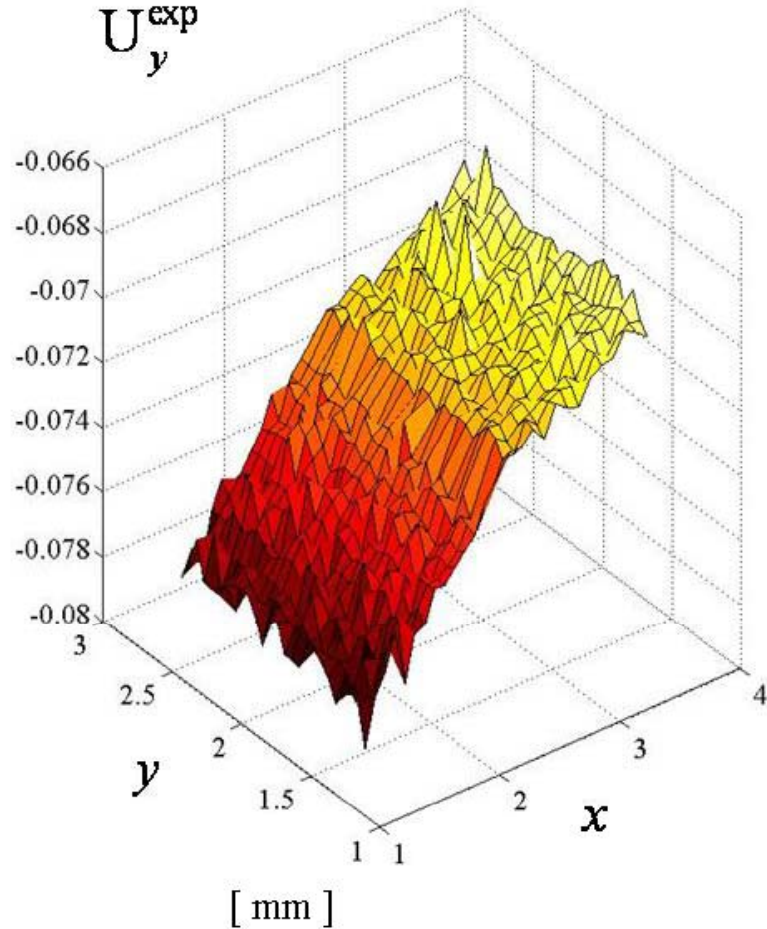


Displacement fields measured by DIC and computed

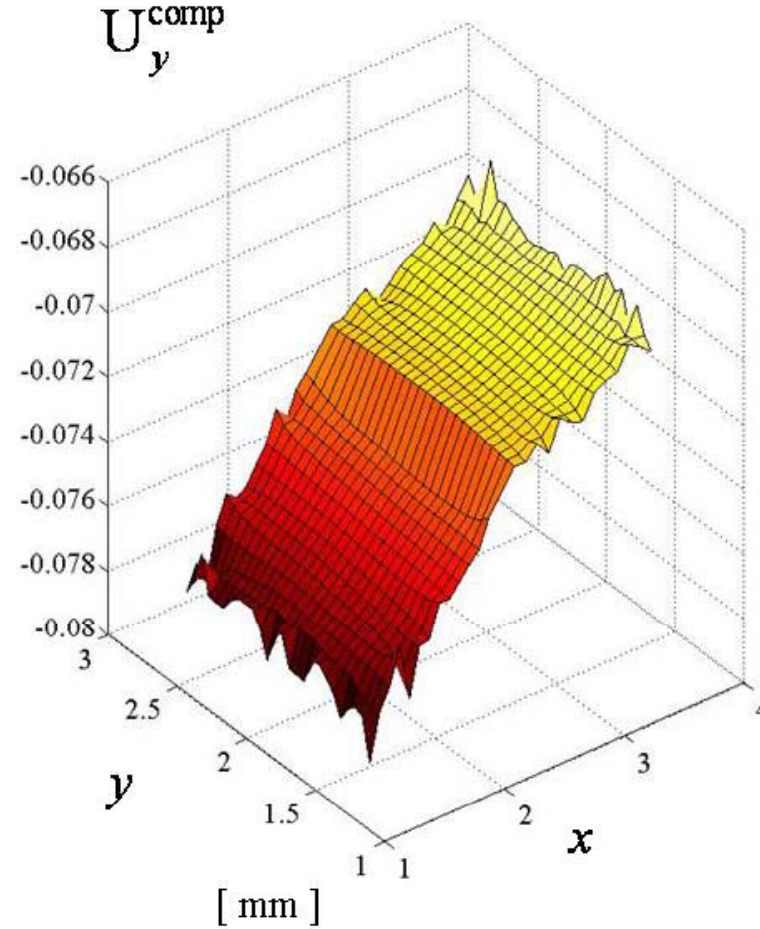
tangential component

$k = 8$

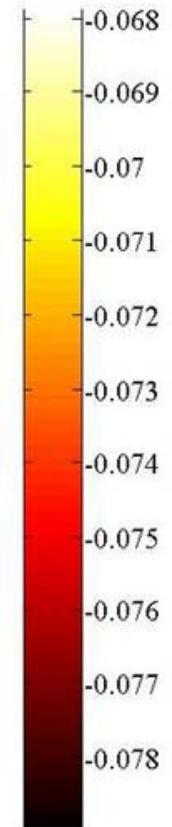
U_y^{exp}



U_y^{comp}



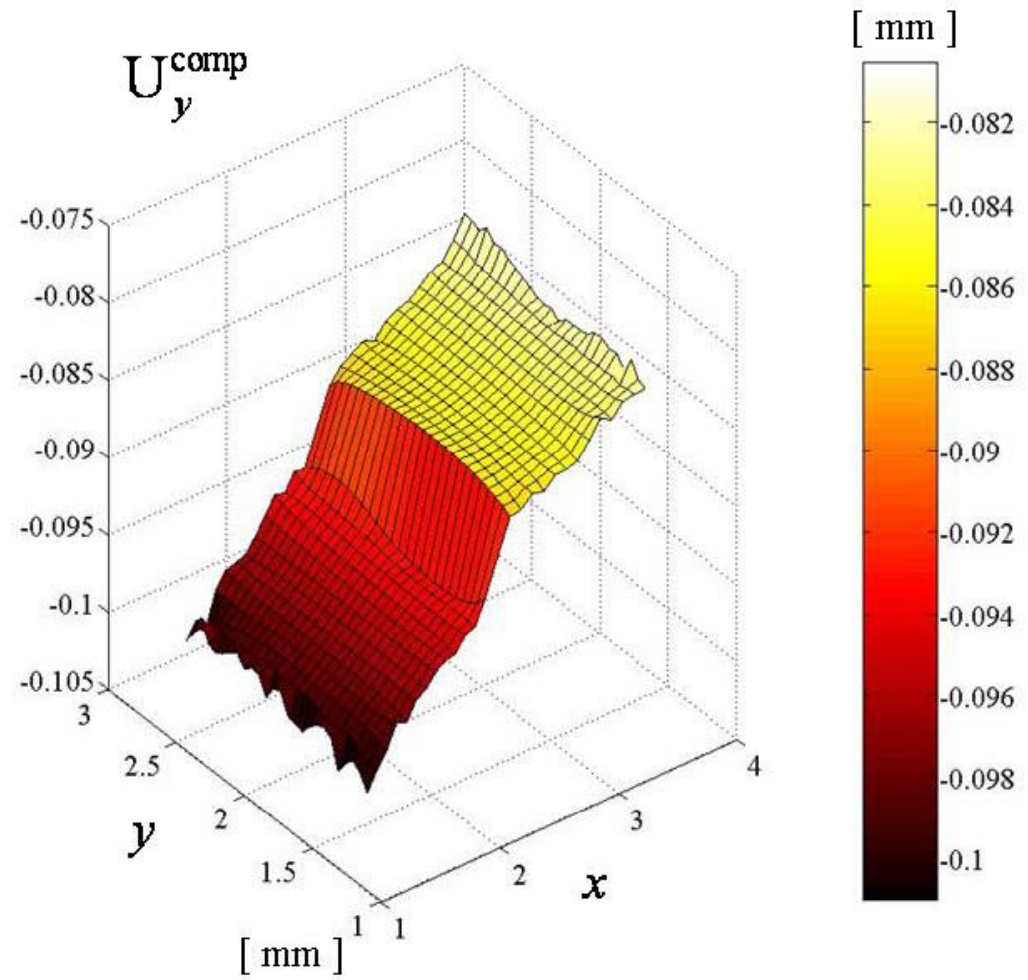
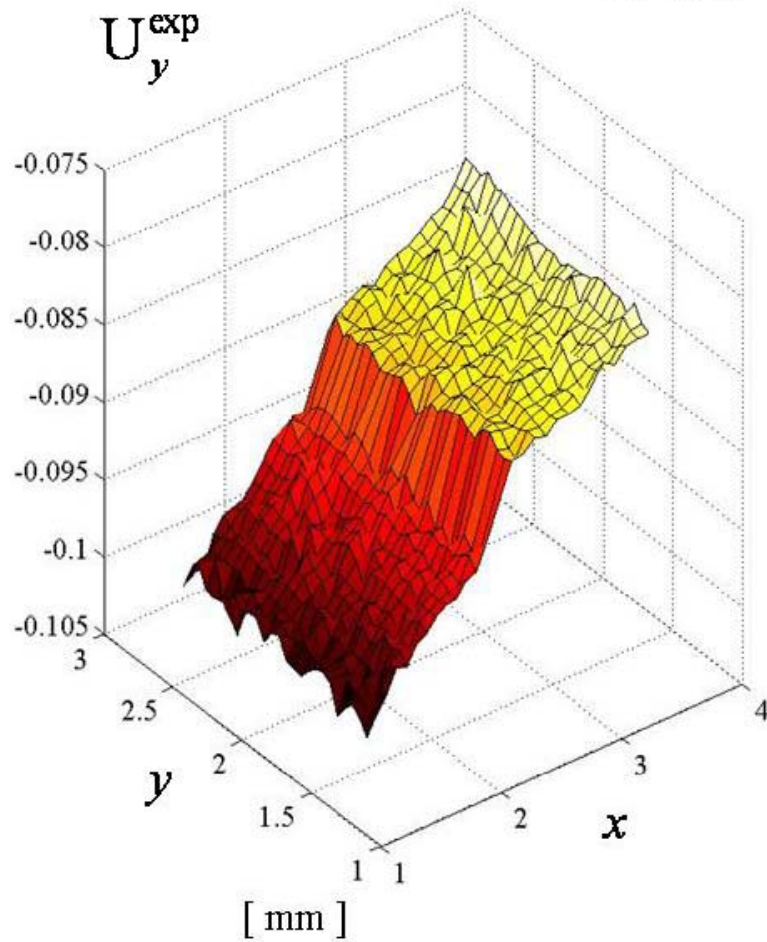
[mm]



Displacement fields measured by DIC and computed

tangential component

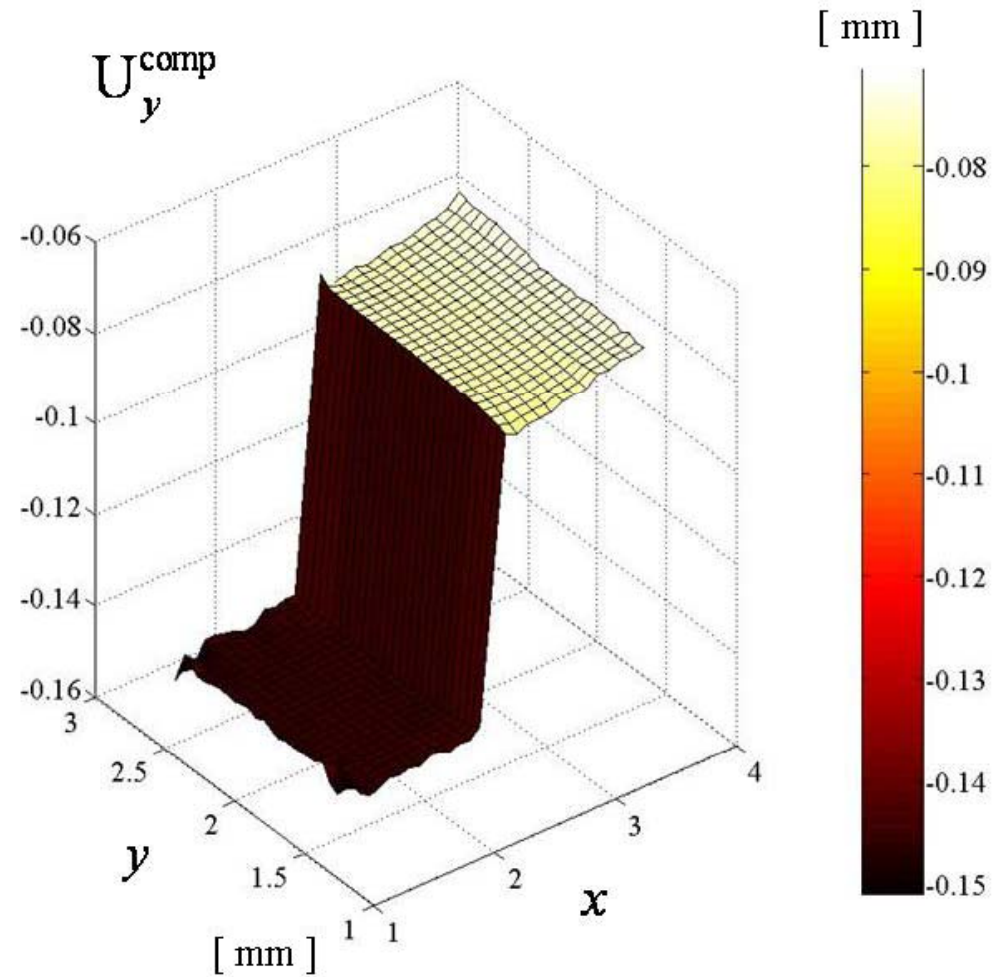
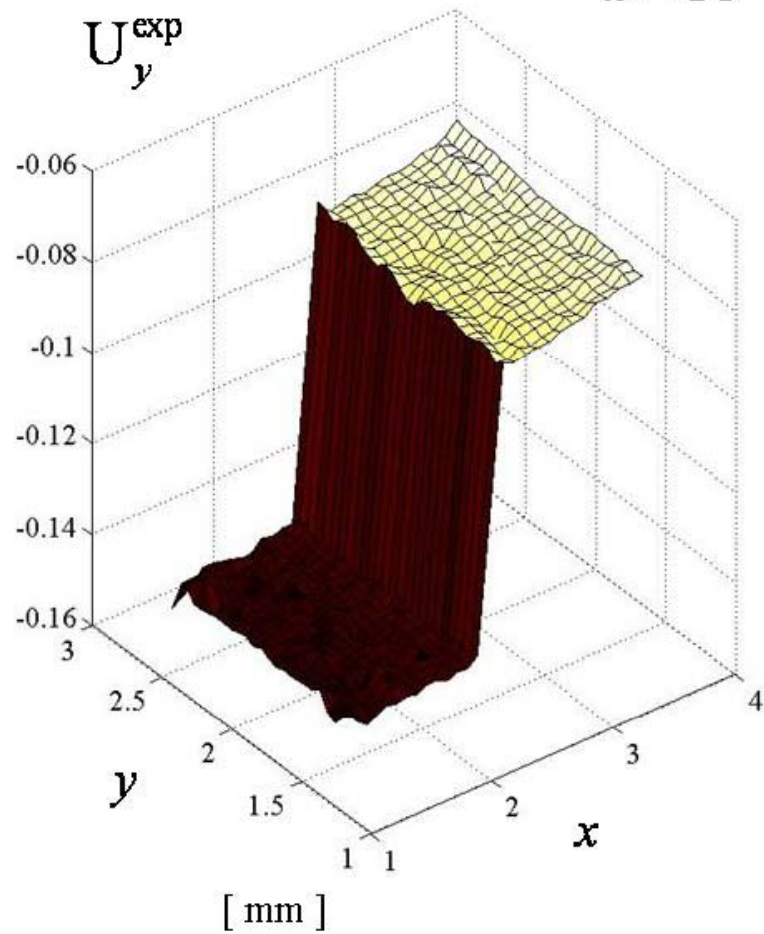
$k = 12$



Displacement fields measured by DIC and computed

tangential component

$k = 16$



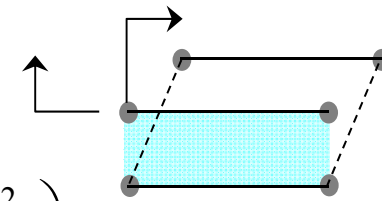
Joint governing parameters to be identified

interface
tractions

“displacement jumps”

Van der Bosch, Schreurs
and Geers, EFM, 2006

$$\begin{cases} p_n = \frac{\phi_n}{\delta_n} \left(\frac{\Delta_n}{\delta_n} \right) \cdot \exp\left(-\frac{\Delta_n}{\delta_n}\right) \cdot \exp\left(-\frac{\Delta_t^2}{\delta_t^2}\right) \\ p_t = \frac{2\phi_t}{\delta_t} \left(\frac{\Delta_t}{\delta_t} \right) \cdot \left(1 + \frac{\Delta_n}{\delta_n}\right) \cdot \exp\left(-\frac{\Delta_n}{\delta_n}\right) \cdot \exp\left(-\frac{\Delta_t^2}{\delta_t^2}\right) \end{cases}$$



$$r = \frac{\phi_t}{\phi_n}$$

assumed a priori

parameters to identify

$$\mathbf{X} = \{\phi_n, \delta_n, \delta_t\}^T$$

$$\hat{\mathbf{X}} = \arg \min_{\mathbf{X}} \left\{ \omega_u(\mathbf{X}) = \sum_{k=1}^{n_t} \mathbf{R}_k^T \mathbf{R}_k \right\}$$

$$\mathbf{R}_k = \mathbf{W}_k^{-1/2} \cdot \left[\mathbf{U}_{\Omega^\circ}^{\text{exp } k} - \mathbf{U}_{\Omega^\circ}^{\text{comp } k}(\mathbf{X} \mid \mathbf{U}_{\partial\Omega}^{\text{exp } k}) \right] \quad k, k = 1, \dots, n_t$$

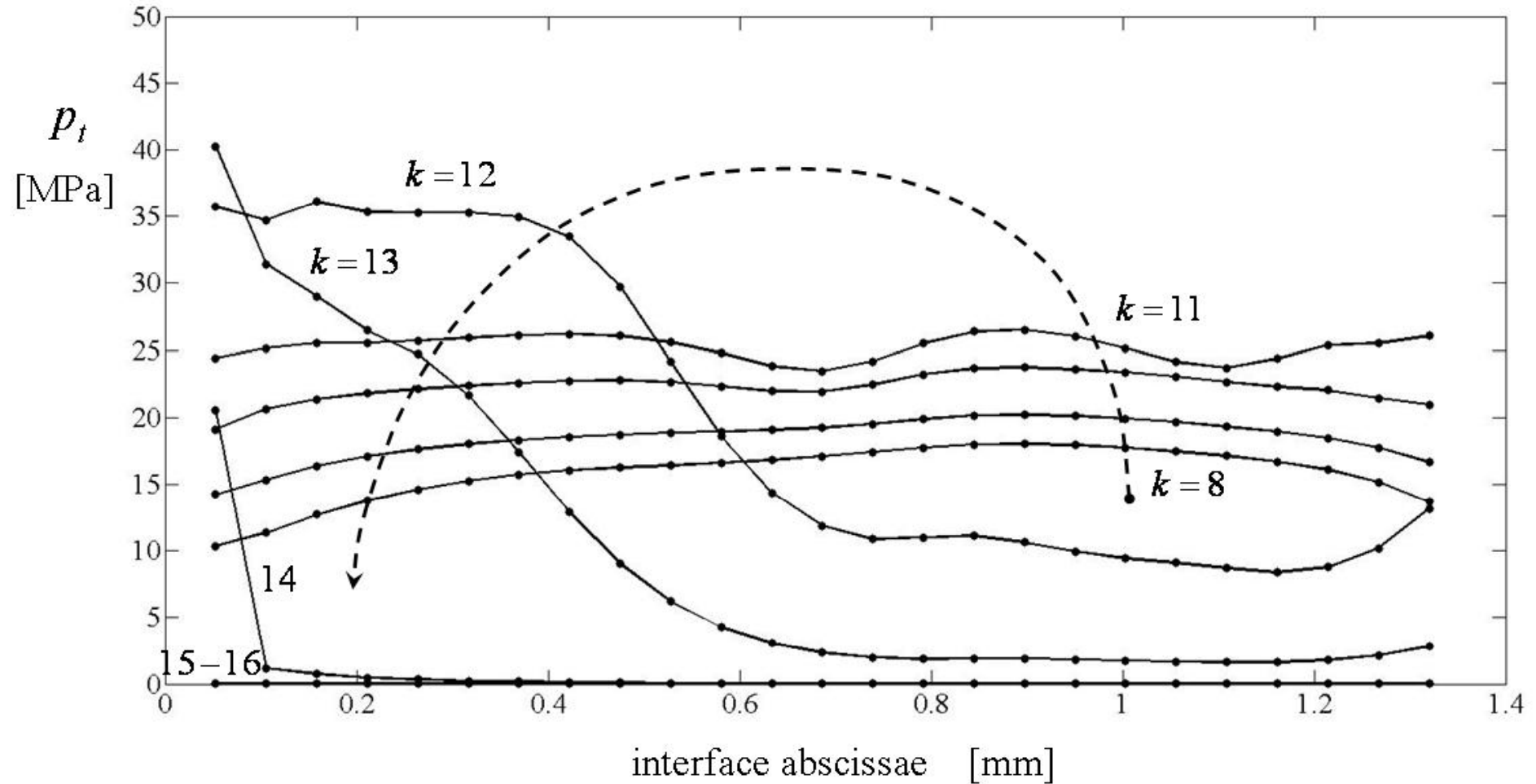
minimization

by Trust Region, reflective,
interior point Method
in a Matlab® environment

boundary conditions provided by DIC

were deterministically prescribed without any regularization provision

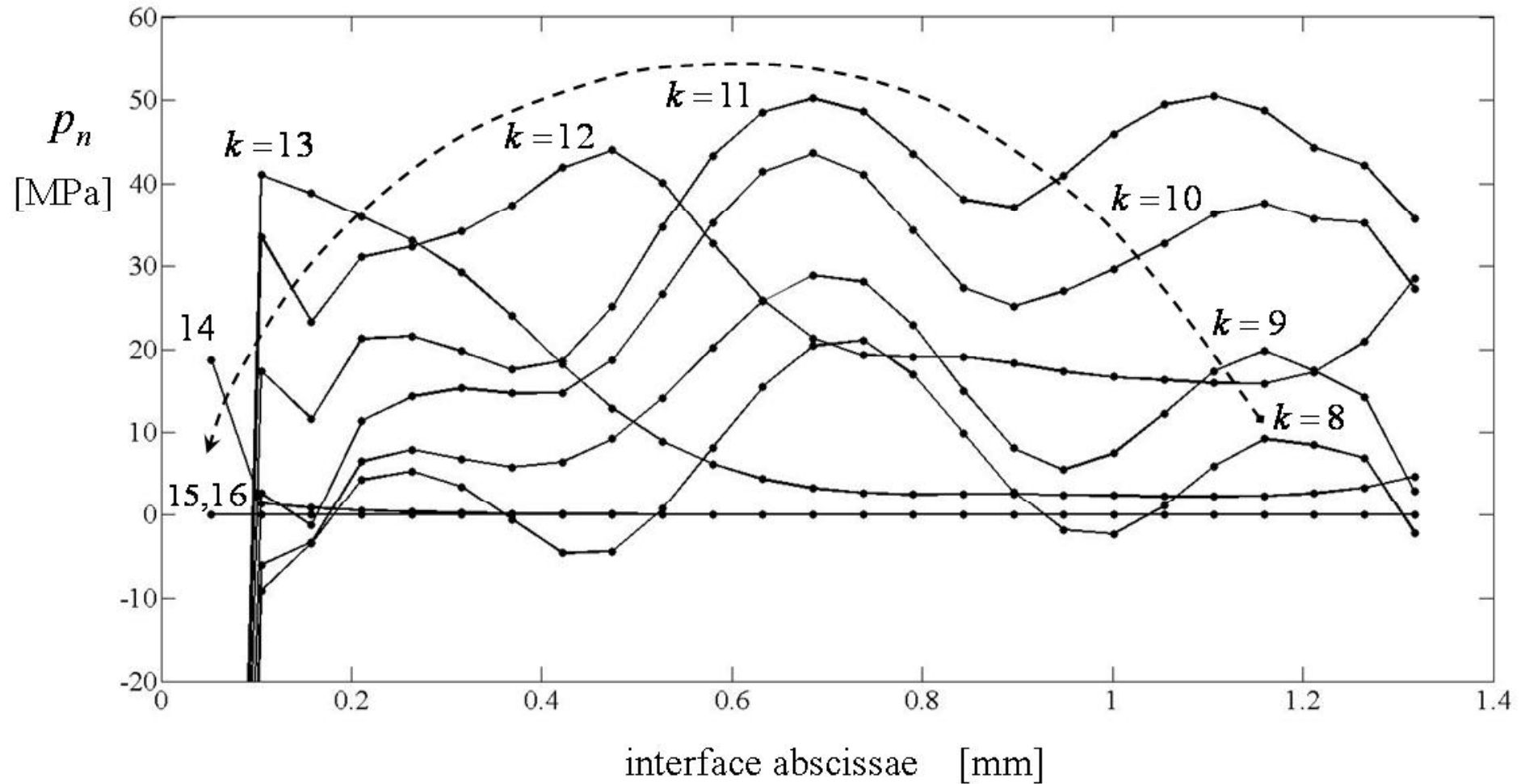
Tangential stress evolution



traction predicted under pure mode II

$$(p_t)_{\max} \simeq 43 \text{ MPa}$$

Normal stress evolution



traction predicted under pure mode I

$$(p_n)_{\max} \simeq 58 \text{ MPa}$$

Closing remarks and future prospects

- DIC measurements especially suitable for calibration/validation of FE models with special reference to :
 - (i) accuracy of 2D/3D geometry assumptions ;
 - (ii) boundary data estimation ;
 - (iii) response of joint/interfaces ;
 - (iv) constitutive relationships .

- Information fusion from several sensors and diverse testing configurations

- Extension to High and Ultra High Temperature (UHT) testing (?)



$$I_1 \equiv \text{tr } \boldsymbol{\sigma} = \sigma_{ij} \delta_{ji} \quad [\text{MPa}]$$

first invariant stress tensor

$$J_2 \equiv \left(\frac{3}{2} S_{ij} S_{ji} \right) \quad [\text{MPa}^2]$$

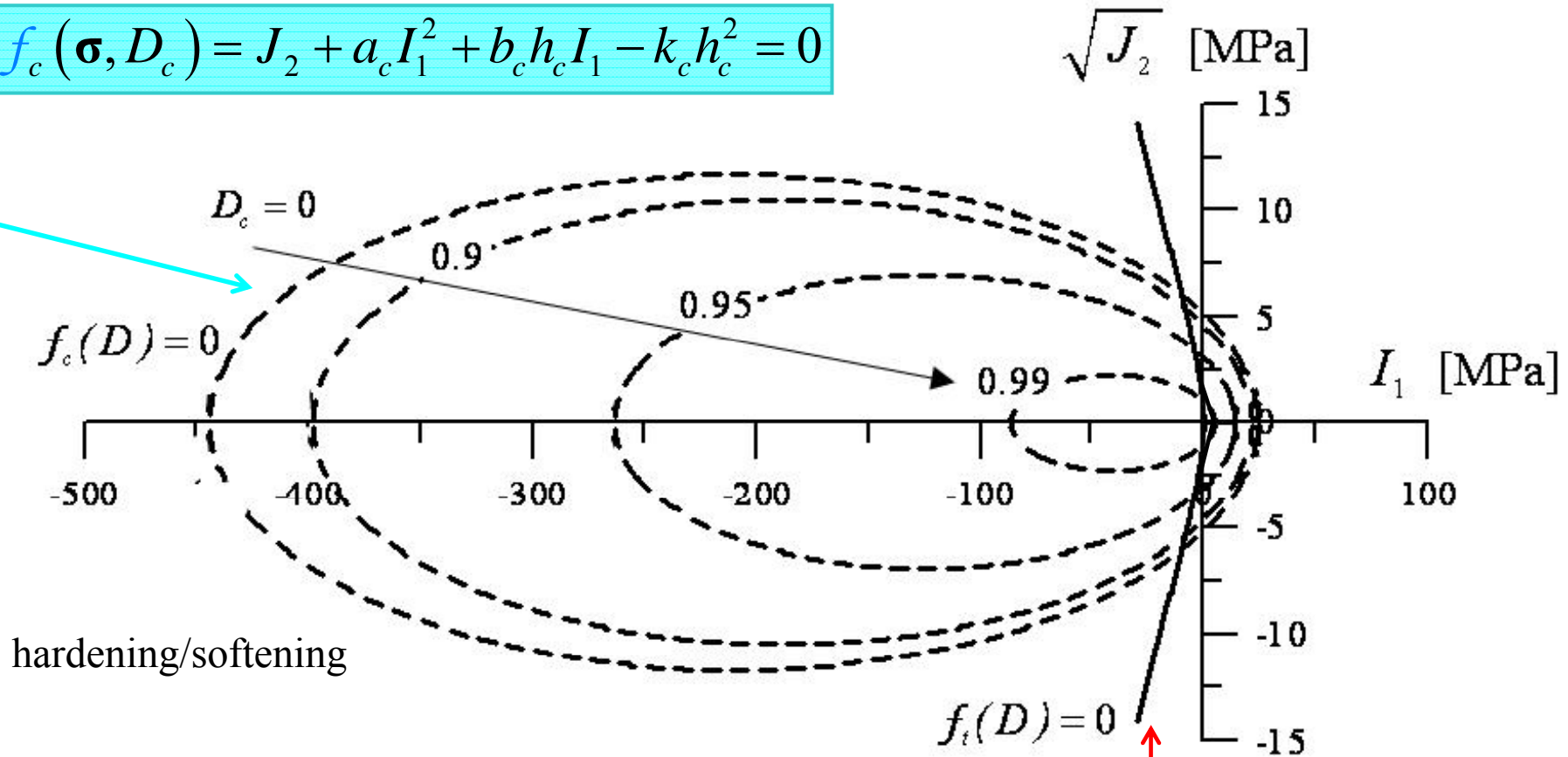
second invariant stress deviator

$$h_i(D_i) \quad (i = t, c)$$

hardening/softening functions

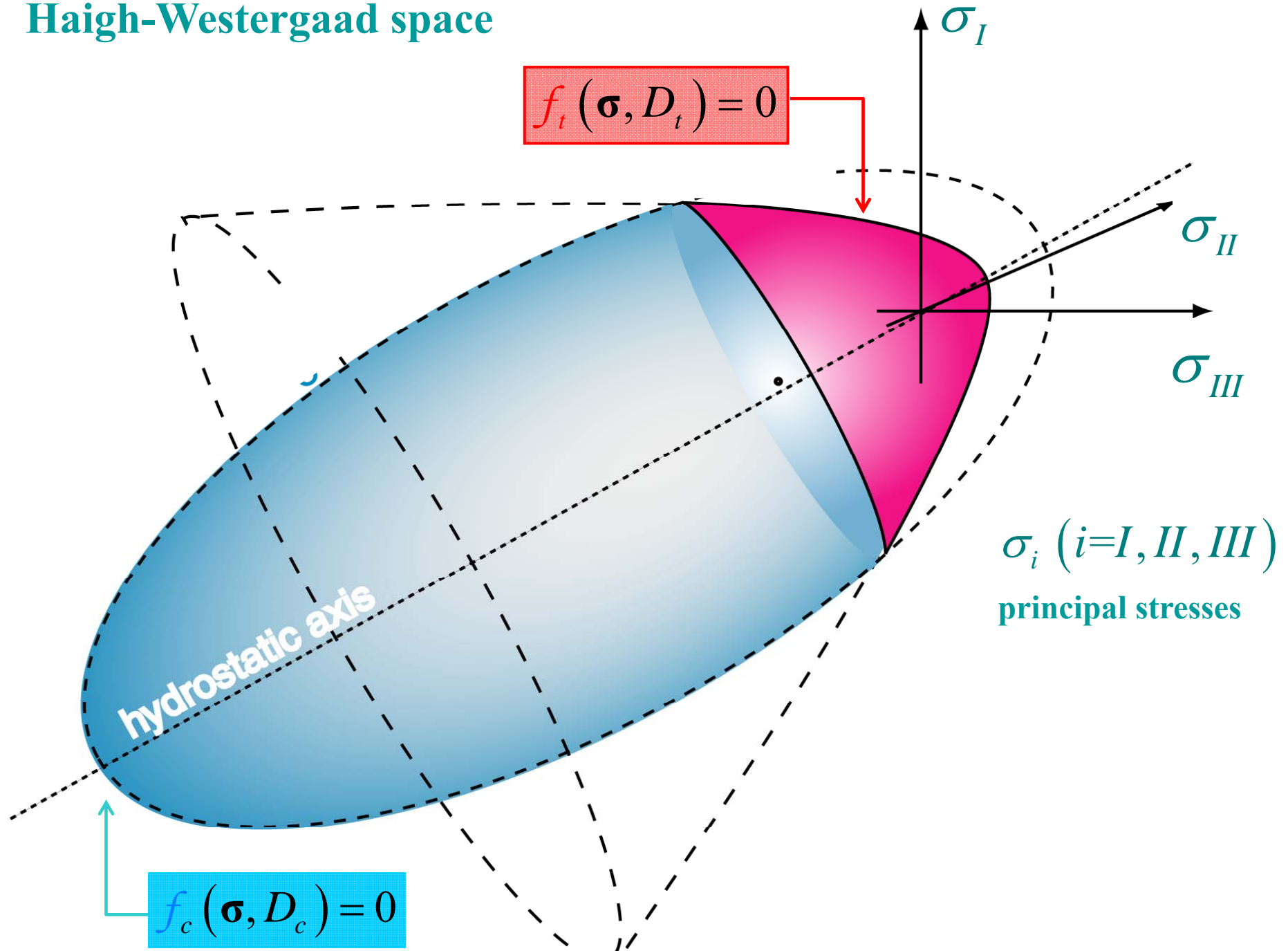
meridian plane

$$f_c(\boldsymbol{\sigma}, D_c) = J_2 + a_c I_1^2 + b_c h_c I_1 - k_c h_c^2 = 0$$



$$f_t(\boldsymbol{\sigma}, D_t) = J_2 - a_t I_1^2 + b_t h_t I_1 - k_t h_t^2 = 0$$

Haigh-Westergaard space

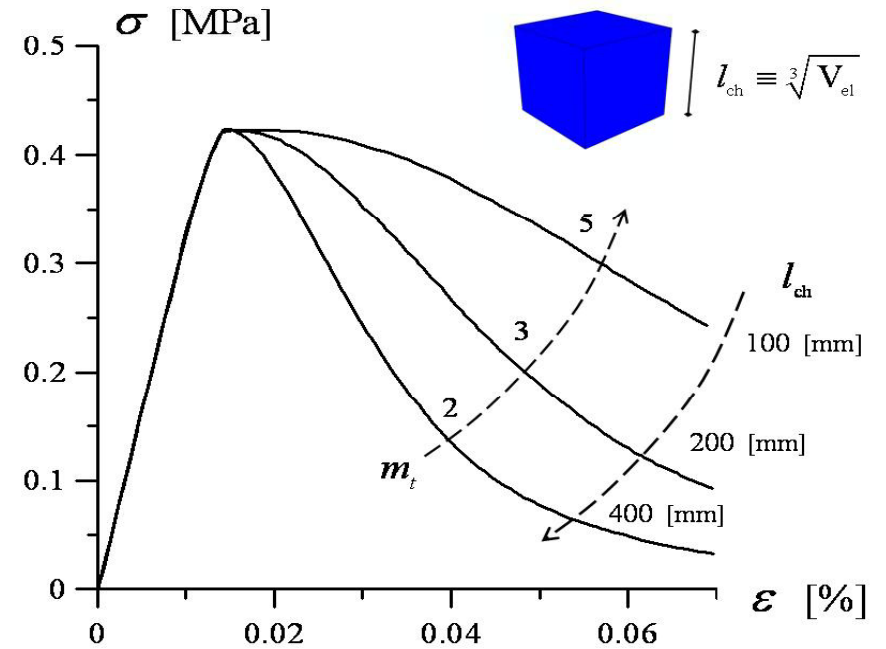
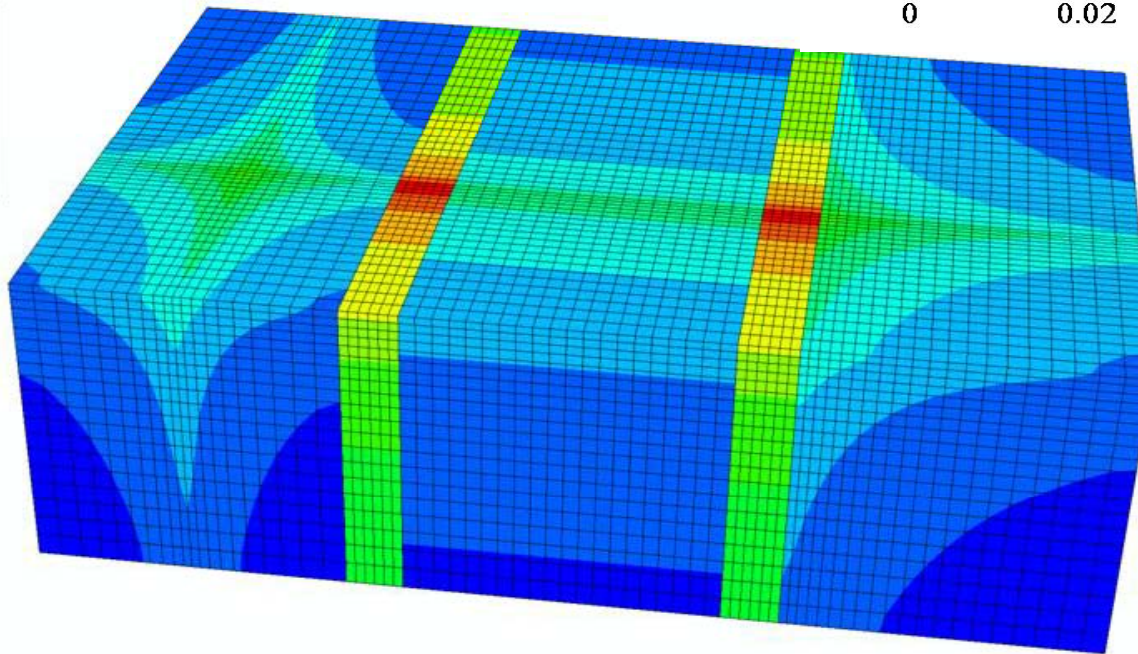
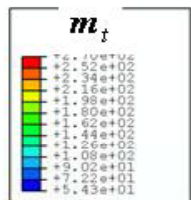


Fracture energy regularization

specific fracture energy

$$g_i(m_i) = \frac{G_i}{l_{ch}^{(e)}}$$

“element” characteristic length



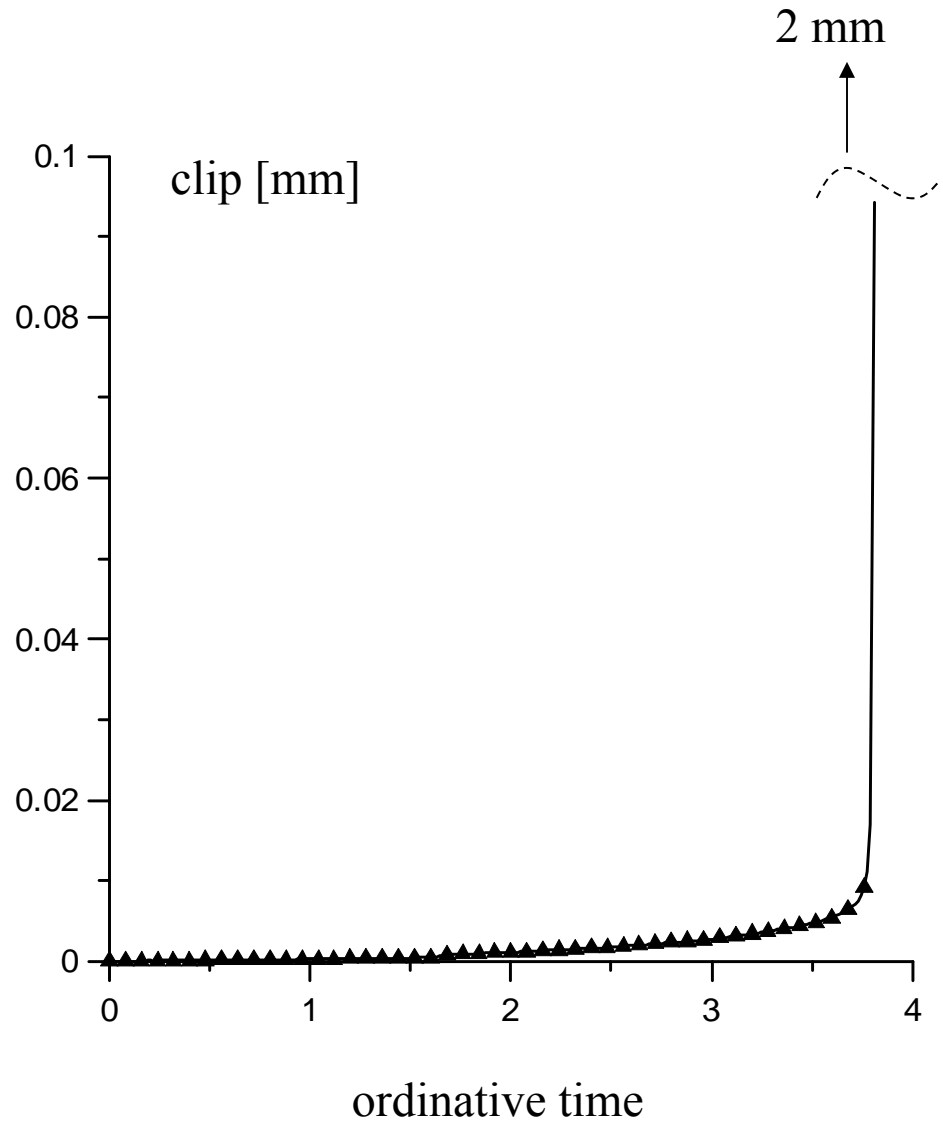
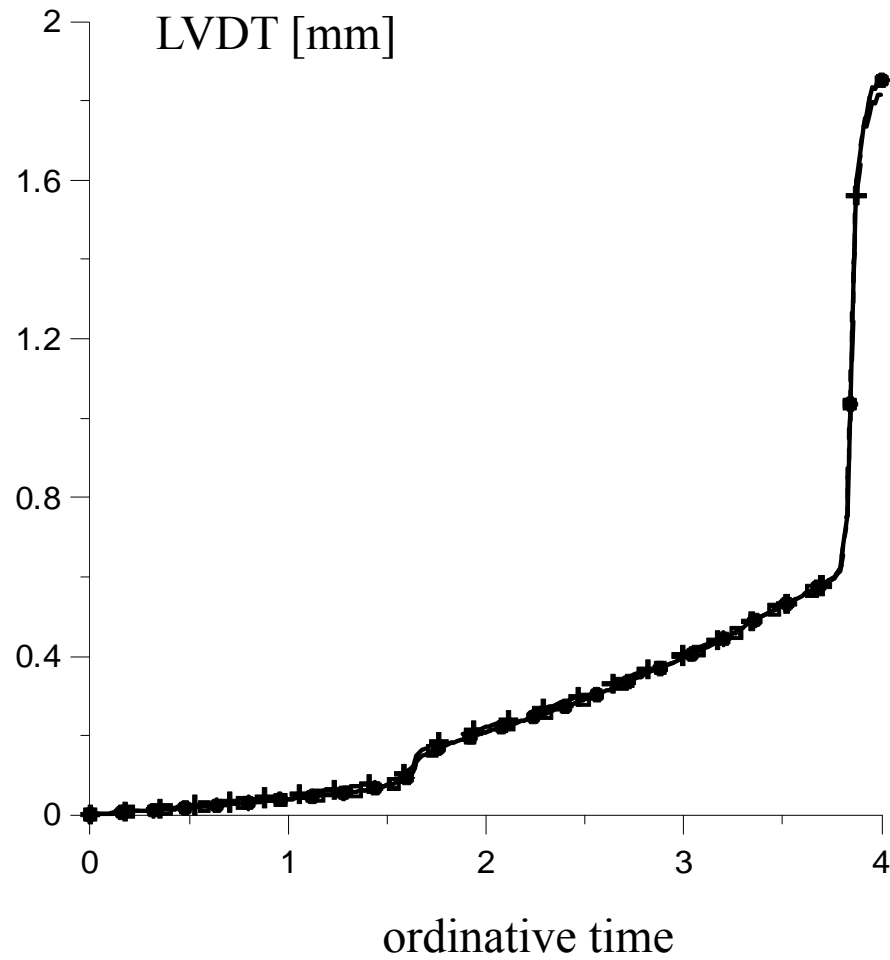
Table

Model parameter	Meaning	Brick	Mortar
E	Young modulus	4250 MPa	5000 MPa
ν	Poisson ratio	0.1	0.1
a_t	parameter governing tensile damage activation function f_t	0.329	0.12
b_t	parameter governing tensile damage activation function f_t	3.78 MPa	2.4 MPa
k_t	parameter governing tensile damage activation function f_t	6.2 MPa ²	10.5 MPa ²
(σ_a/σ_{ot})	uniaxial stress at the elastic limit / uniaxial peak stress, in tension	0.8	0.8
D_{ot}	tensile damage at peak	0.1	0.1
a_c	parameter governing compressive damage activation function f_c	0.0025	0.0025
b_c	parameter governing compressive damage activation function f_c	2.75 MPa	1.1 MPa
k_c	parameter governing compressive damage activation function f_c	36 MPa ²	28 MPa ²
$(\sigma_{ec}/\sigma_{oc})$	uniaxial stress at the elastic limit / uniaxial peak stress, in compression.	0.7	0.7
D_{oc}	compressive damage at peak	0.3	0.3
G_t	fracture energy in tension	0.14 N/mm	0.09 N/mm
G_c	fracture energy in compression	14 N/mm	9 N/mm

Table 1: Damage model parameters adopted for the historical bricks and high-strength mortar joints.

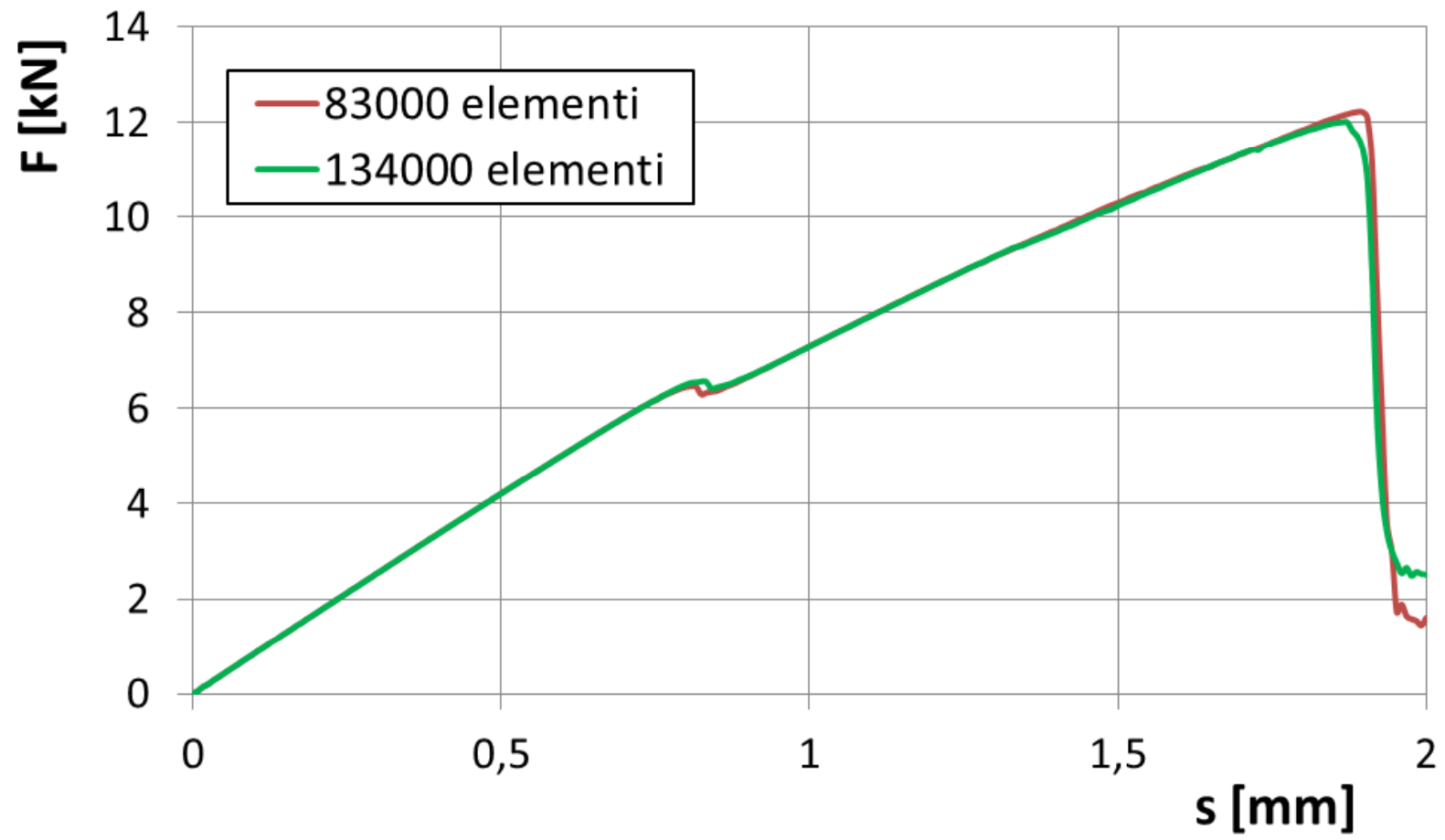


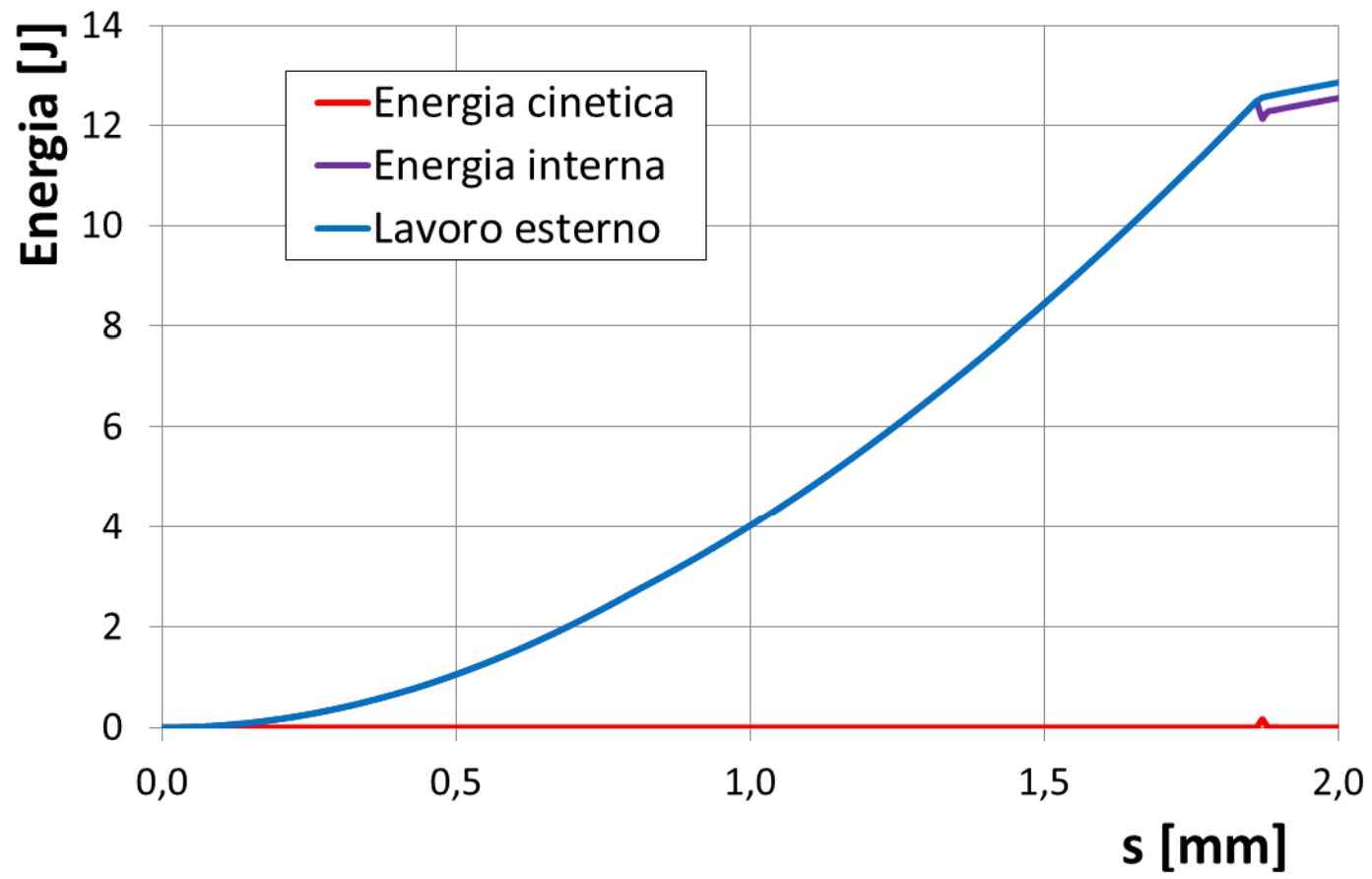
Local predictions of FE model



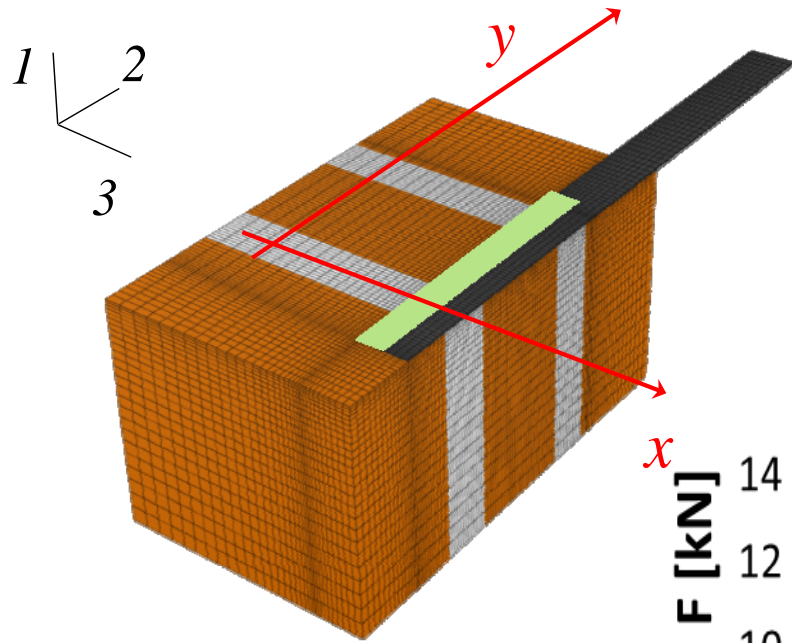
Closing remarks

- Single-lap shear tests were performed under clip-control
- Delamination of CFRP strips from a small masonry pillar was simulated under the hypothesis of a perfect adhesion.
- 3D heterogeneous FE model with elastic-damageable phases was developed
- Optical monitoring was validated with correction of optical distortion
- Information fusion
- Future prospects: combining interface and bulk damage

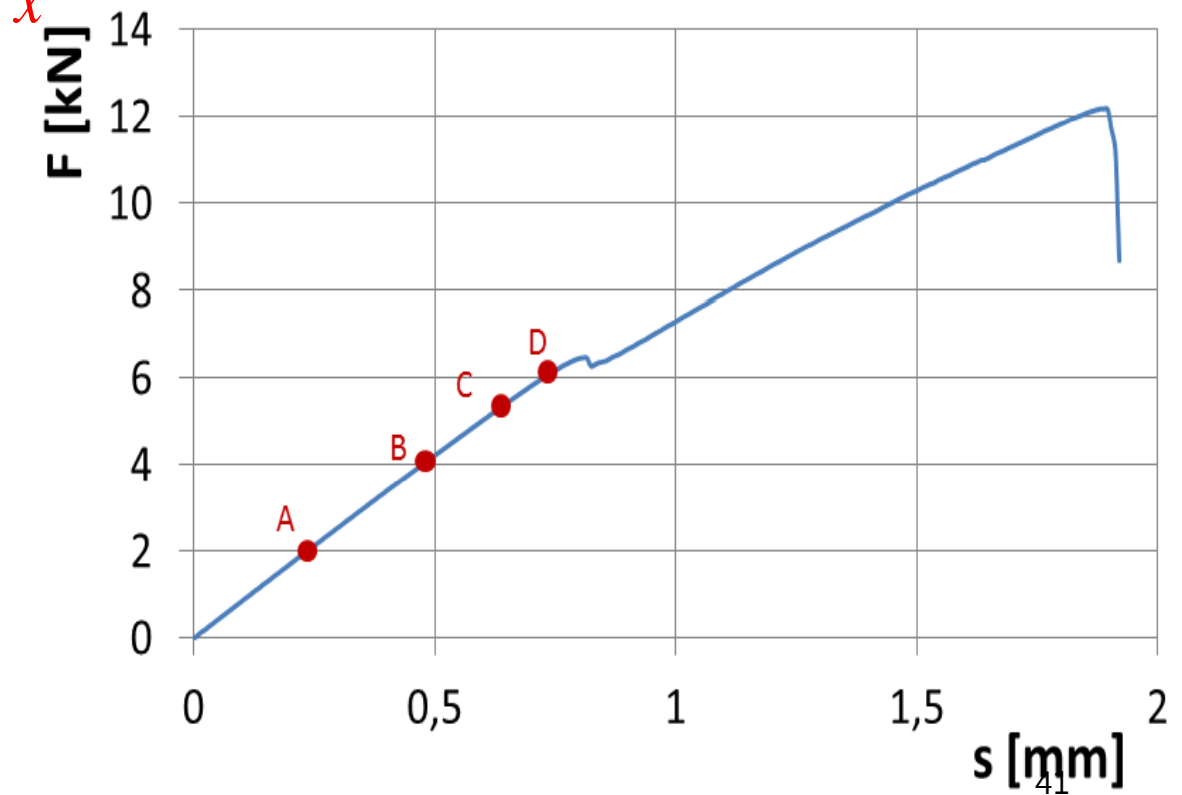




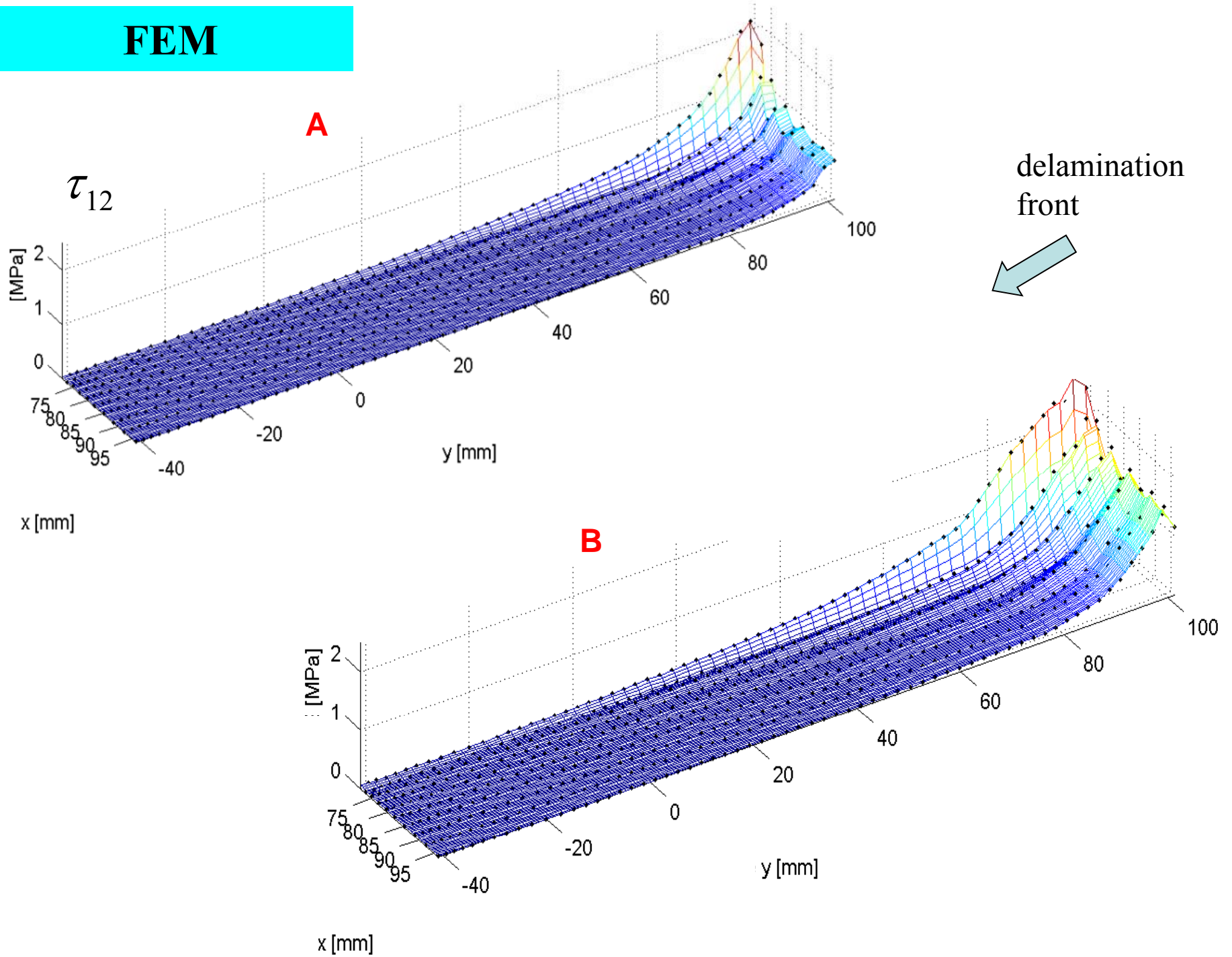
Local traction predicted by FEM

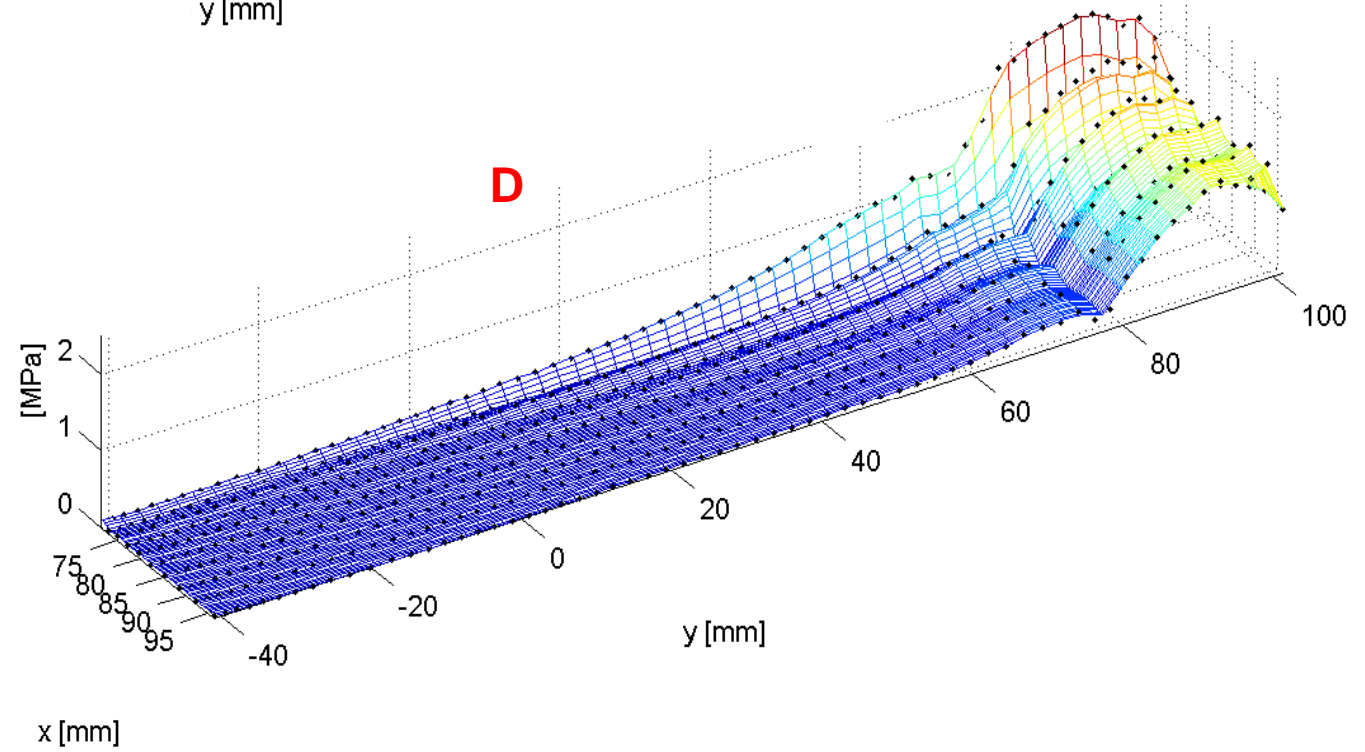
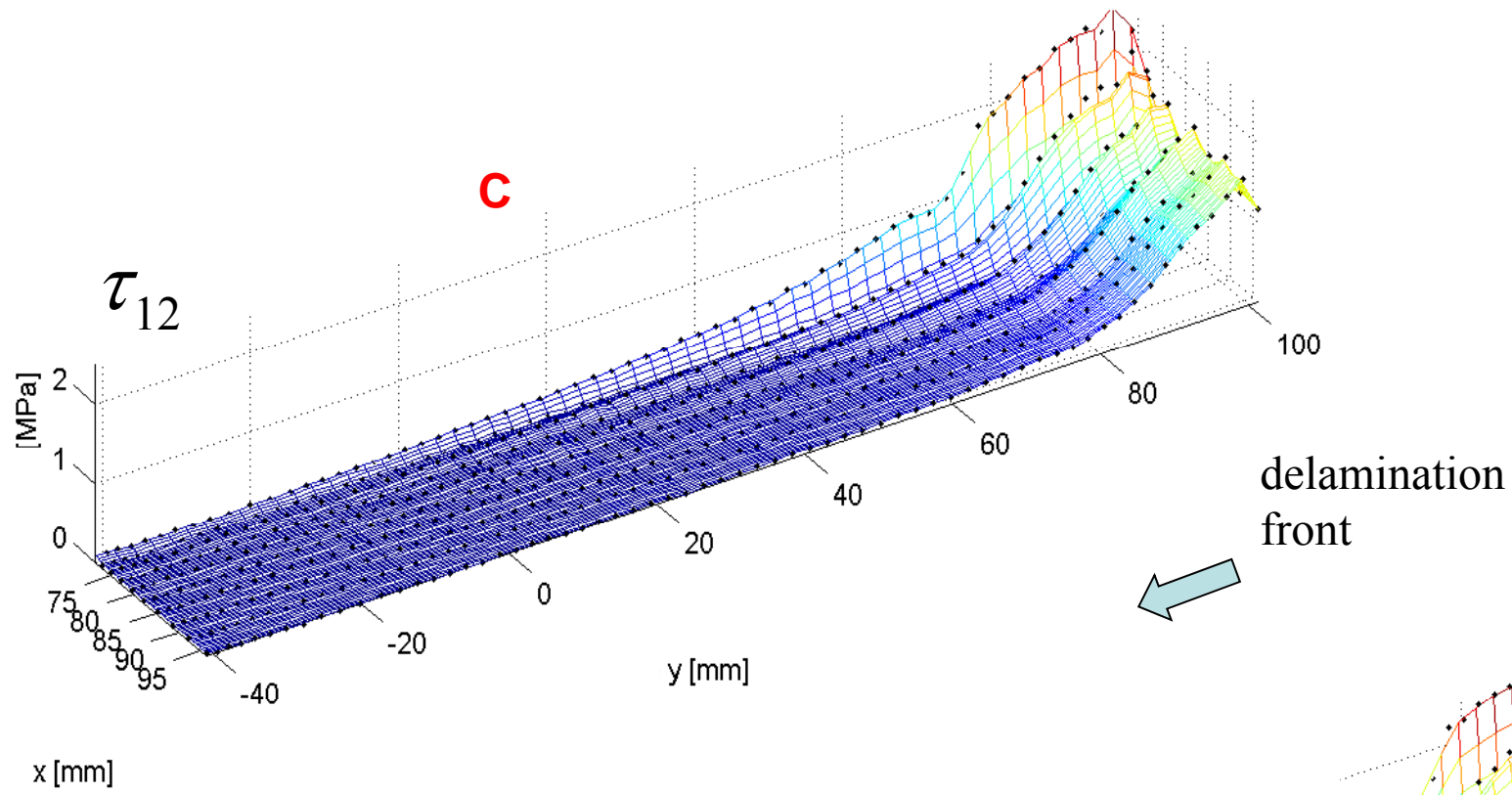


$$\bar{\tau} = \frac{F_{max}}{A} \approx 1.7 \text{ MPa}$$



FEM





A priori assumed parameters (“diffuse load cell”)

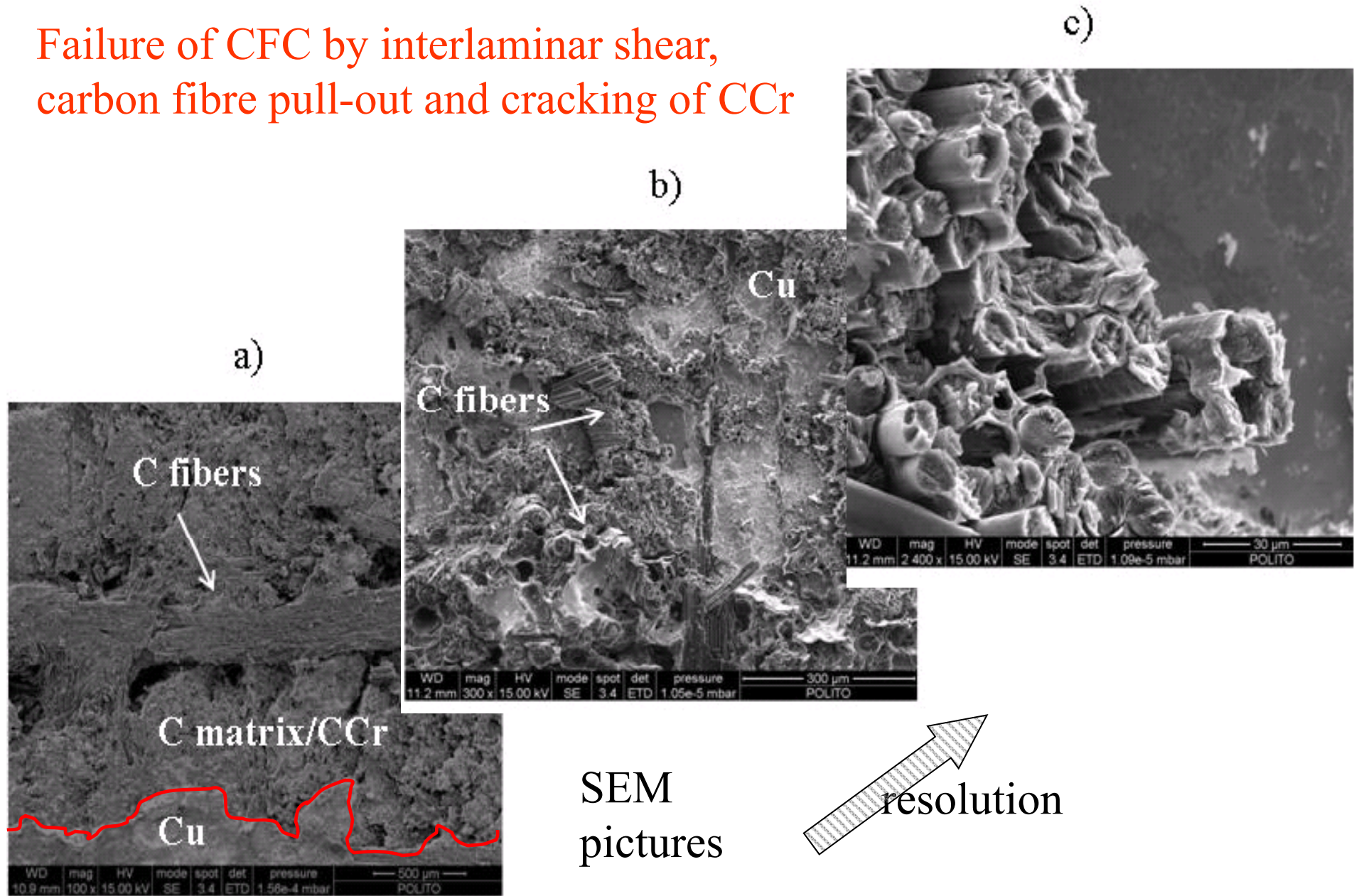
Ceramic phase CFC SEP NB31: Mechanical parameters at room temperature		
Elastic properties	Hill parameters for plastic anisotropy	Ramberg-Osgood parameters for the incompressible strains
$E_x = 107$ [GPa]; $E_y = 15$ [GPa]; $E_z = 12$ [GPa]; $\nu_{xy} = 0.10$; $\nu_{xz} = 0.20$; $\nu_{yz} = 0.20$; $G_{xy} = 10$ [GPa]	$F = 0.8$; $G = 0.5$; $H = 0.5$; $N = 10$;	$\sigma_0 = 100$ [MPa]; $E_R / \alpha = 2083$ [GPa]; $n = 7$;

Cu phase: Mechanical parameters at room temperature	
Elastic properties	Ramberg-Osgood parameters
$E = 125$ [GPa]; $\nu = 0.34$;	$\sigma_0 = 300$ [MPa]; $\alpha = 0.06$; $n = 7$;

Ref: ITER Final Report, Material Assessment Report, 2001

Post-mortem analyses of fracture surface by SEM

Failure of CFC by interlaminar shear, carbon fibre pull-out and cracking of CCr



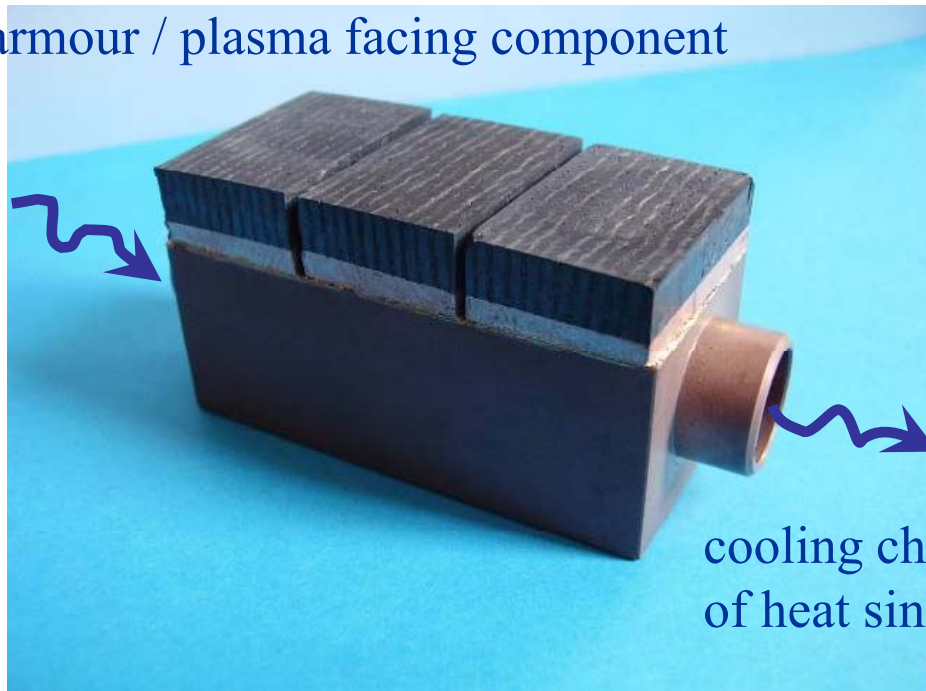
ITER Final Design Report (July 2001): Materials

Table 2.3-3 Properties of SEP NB31 [11]

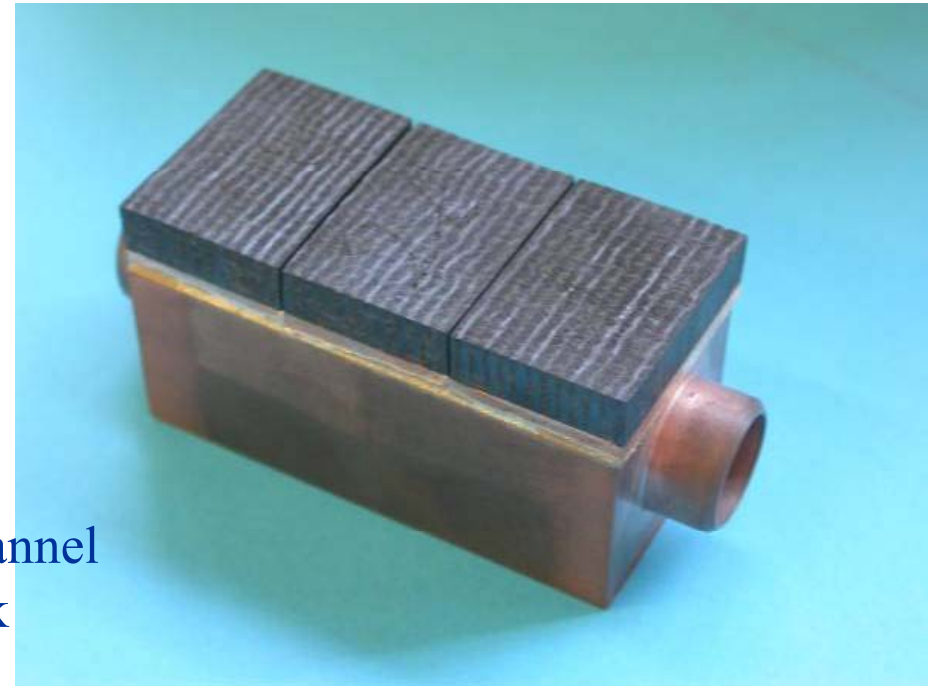
Properties	T, °C	x	y	z
Thermal conductivity, W/mK	RT	323	117	115
	800	154	58	55
	1000	145	56	52
	1500	136	55	51
Specific Heat, J/kg K	RT	780		
	800	1820		
	1000	2000		
CTE, 10^{-6} K^{-1}	800	0.4	1	2.1
	1000	0.5	1.2	2.7
Tensile strength, MPa	RT	130	30	19
Tensile strain, %	RT	0.14	0.30	/
Young Modulus, GPa	RT	107	15	/
Poisson's ration	RT	xz: 0.2 xy: 0.1		yz: 0.2 yx: 0.1
Compressive strength, MPa	RT	102	31	/
Shear strength, MPa	RT	xz: 15		yz: 9
Electrical resistivity, $\mu\Omega\text{m}$	RT	3.7	12.4	
Density, kg/m^3	RT	1900		
Porosity, %	RT	8		

Flat-tile mock-ups manufacturing

armour / plasma facing component



cooling channel
of heat sink



CFC NB31/Cu/CuCrZr

- 1) High heat flux applied on CFC surface
up to 10 MW/m^2 (3000 cycles)
up to 20 MW/m^2 during transient events (20 cycles)
- 2) Neutron irradiation and radiation damage

Problems when joining metals and CFC

- Large thermal expansion mismatch

between Cu and CFC

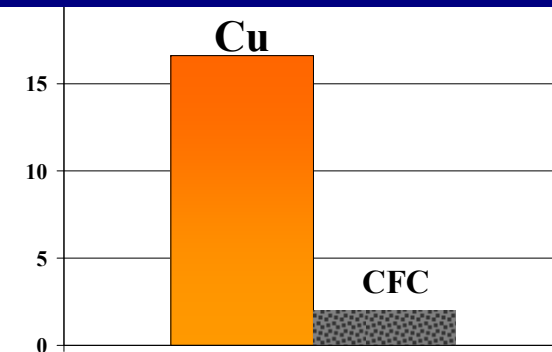
$$\alpha_{\text{CFC}} = 1,7 - 3,3 \times 10^{-6} \text{ K}^{-1},$$

$$\alpha_{\text{Cu}} = 16,6 \times 10^{-6} \text{ K}^{-1}$$

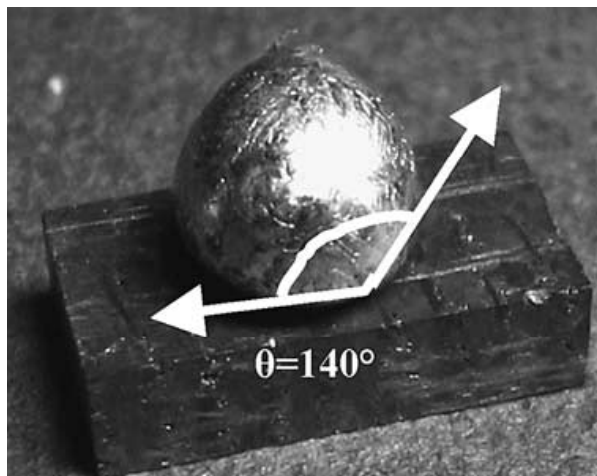
$\times 10^{-6} \text{ K}^{-1}$

→ high residual stresses

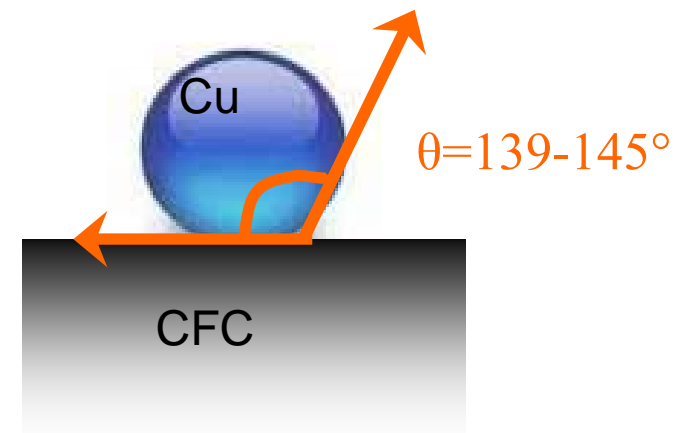
Thermal expansion at 300 °C



- Low wettability of molten copper on CFC (contact angle= 140°)



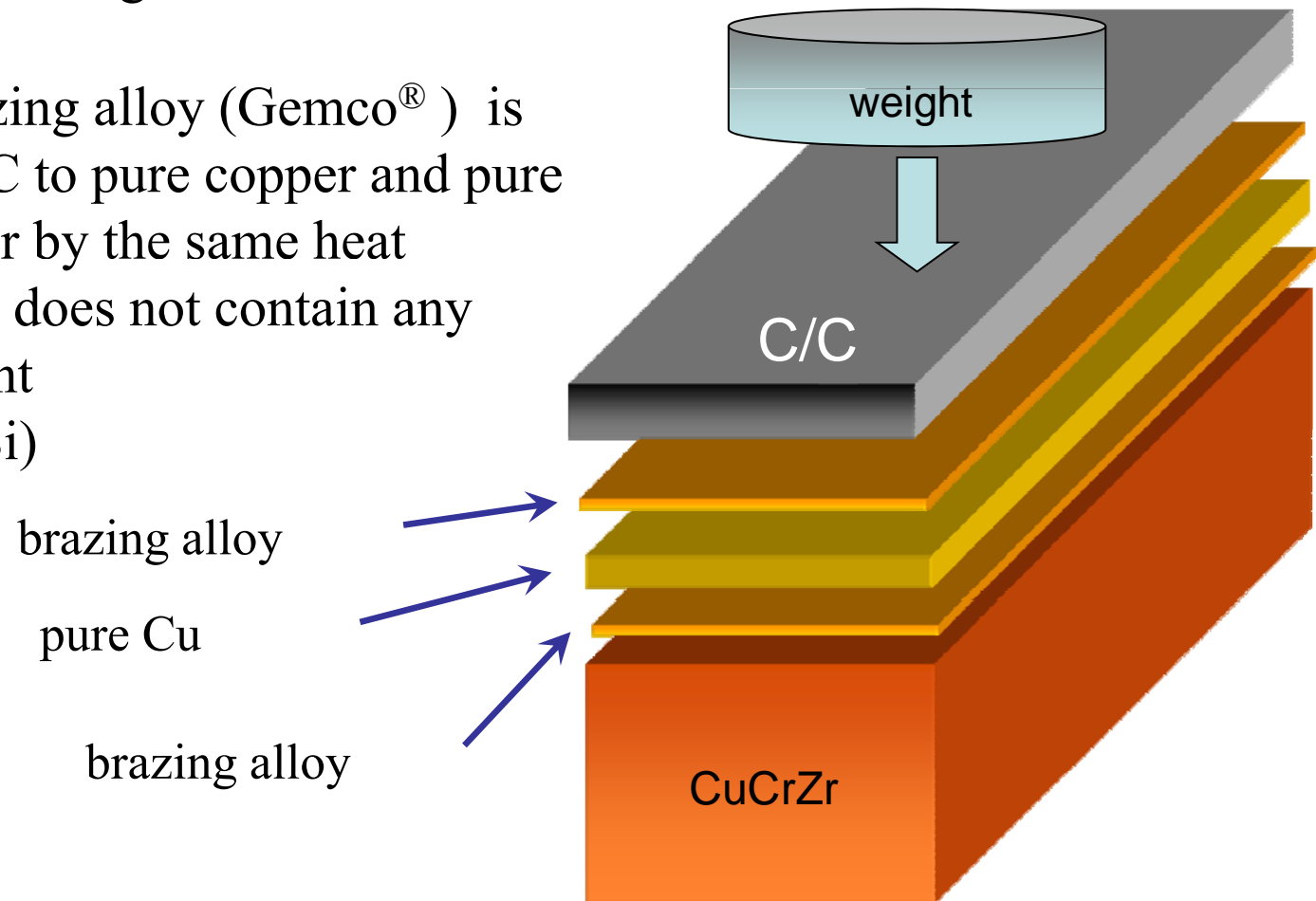
sessile drop test
at 1100 °C
for 30 min
under Argon



Joint manufacturing by one-step brazing

1. Composite surface is modified by direct reaction with chromium → a carbide layer is formed: large reduction of the C/C-Cu contact angle.
2. Commercial brazing alloy (Gemco[®]) is used to braze C/C to pure copper and pure copper to CuCrZr by the same heat treatment. Alloy does not contain any activating element (such as Ti and Si)

Ferraris et alii, J. Nucl Mat.
2008



2D- Digital Image Correlation

“passive” advection of the local texture
(optical flow conservation)

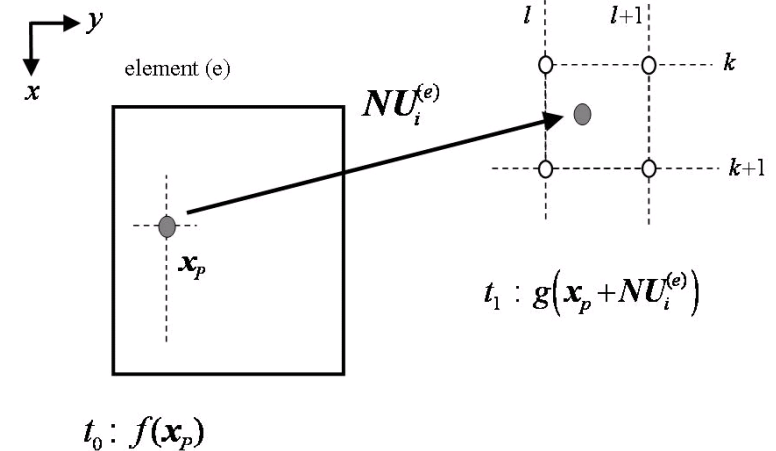
✓ local form

reference image

$$f(\mathbf{x}) \simeq g(\mathbf{x} + \mathbf{u}[\mathbf{x}])$$

deformed image

displacement vector



✓ weak form

similarity measure

$$\arg \min_{\mathbf{u}} \left\{ \eta(\mathbf{u}) = \int_{\Omega} [f(\mathbf{x}) - g(\mathbf{x} + \mathbf{u})]^2 d\mathbf{x} \right\}$$

variational problem

$$\mathbf{u}_{i+1} = \mathbf{u}_i + \delta \mathbf{u}_{i+1} \text{ incremental form}$$

Truncated expansion and stationarity

Euler-Lagrange stationarity condition $\delta\eta_2 = \langle \text{grad } \eta_2, \delta\mathbf{v}_{i+1} \rangle = 0 \quad \forall \delta\mathbf{v}_i \in \mathbf{L}_2$

$$a(\delta\mathbf{u}_{i+1}, \delta\mathbf{v}_{i+1}) = 2 \int_{\Omega} \delta\mathbf{v}_{i+1}^T \nabla g \cdot \nabla g^T \delta\mathbf{u}_{i+1} d\mathbf{x} \quad \mathcal{K} = \nabla g \cdot \nabla g^T$$

bilinear form

$$F(\delta\mathbf{v}_{i+1}) = 2 \int_{\Omega} \delta\mathbf{v}_{i+1}^T \nabla g [g(\mathbf{x} + \mathbf{u}_i) - f(\mathbf{x})] d\mathbf{x}$$

linear form

$$\mathbf{L}_2(\Omega) \equiv L_2 \times L_2 \times L_2$$

$$\text{find } \delta\mathbf{u}_i \in \mathbf{L}_2 : a(\delta\mathbf{u}_i, \delta\mathbf{v}_i) = F(\delta\mathbf{v}_i) \quad \forall \delta\mathbf{v}_i \in \mathbf{L}_2$$

semi-coercive variational problem

multiplicity of solution $\mathbf{u}_0 + \text{Ker } \mathcal{K}$

Galerkin finite-element discretization

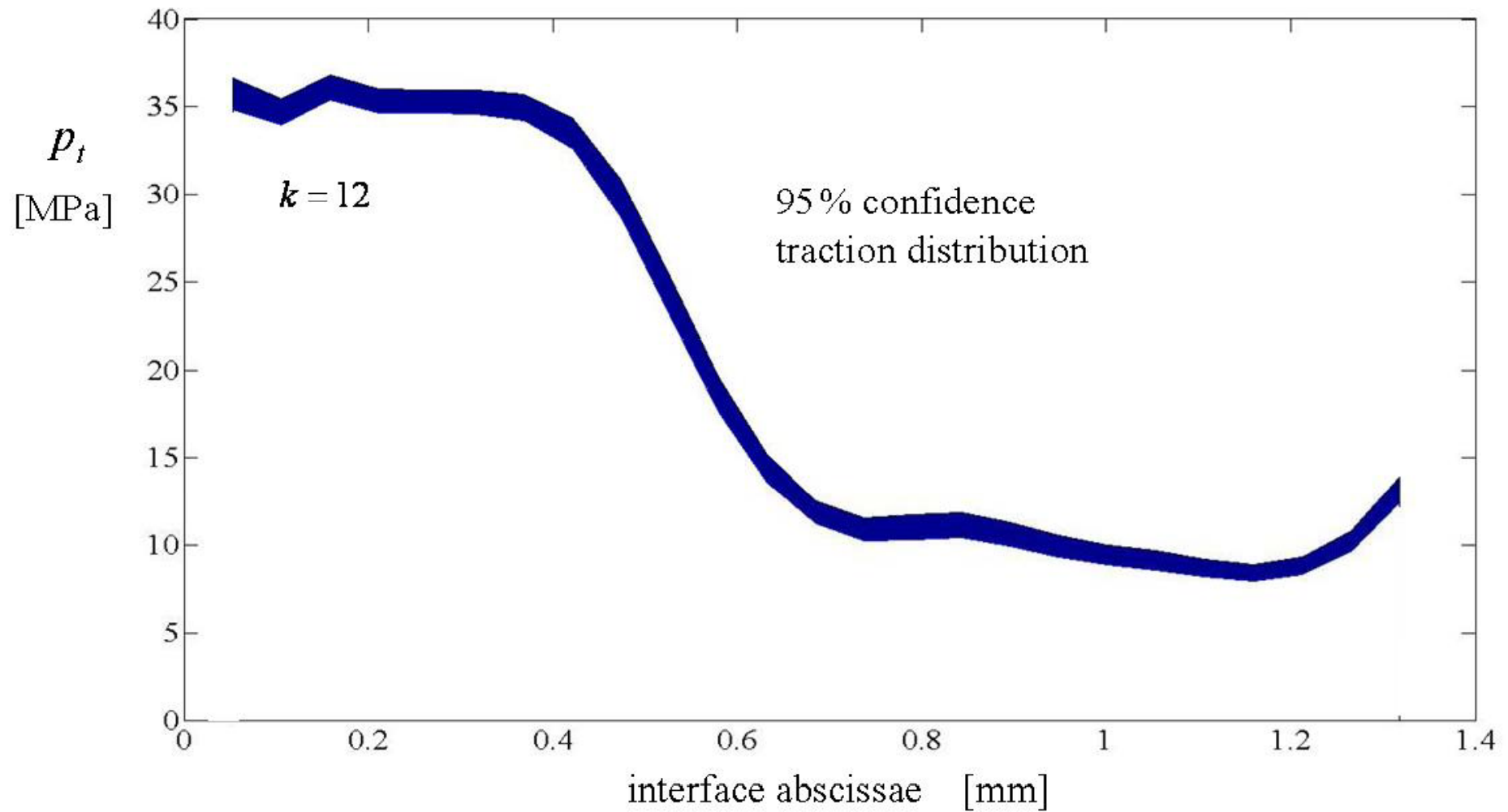
$$\mathbf{u} = \mathbf{N}\mathbf{U}(\mathbf{e})$$

pseudo-stiffness

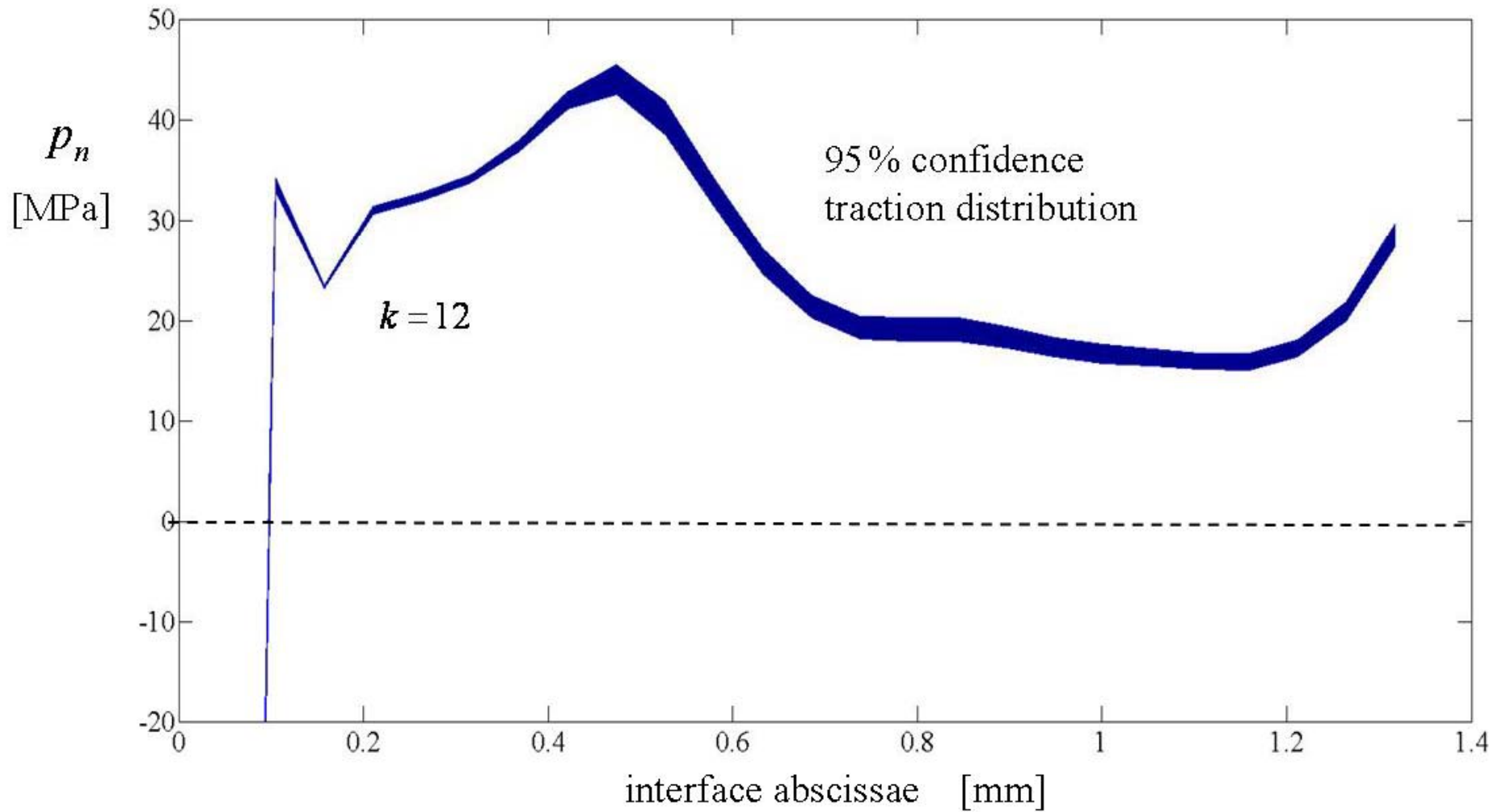
$$\mathbf{K} \cdot \delta\mathbf{U}_{i+1} = \mathbf{B}$$

pseudo-load

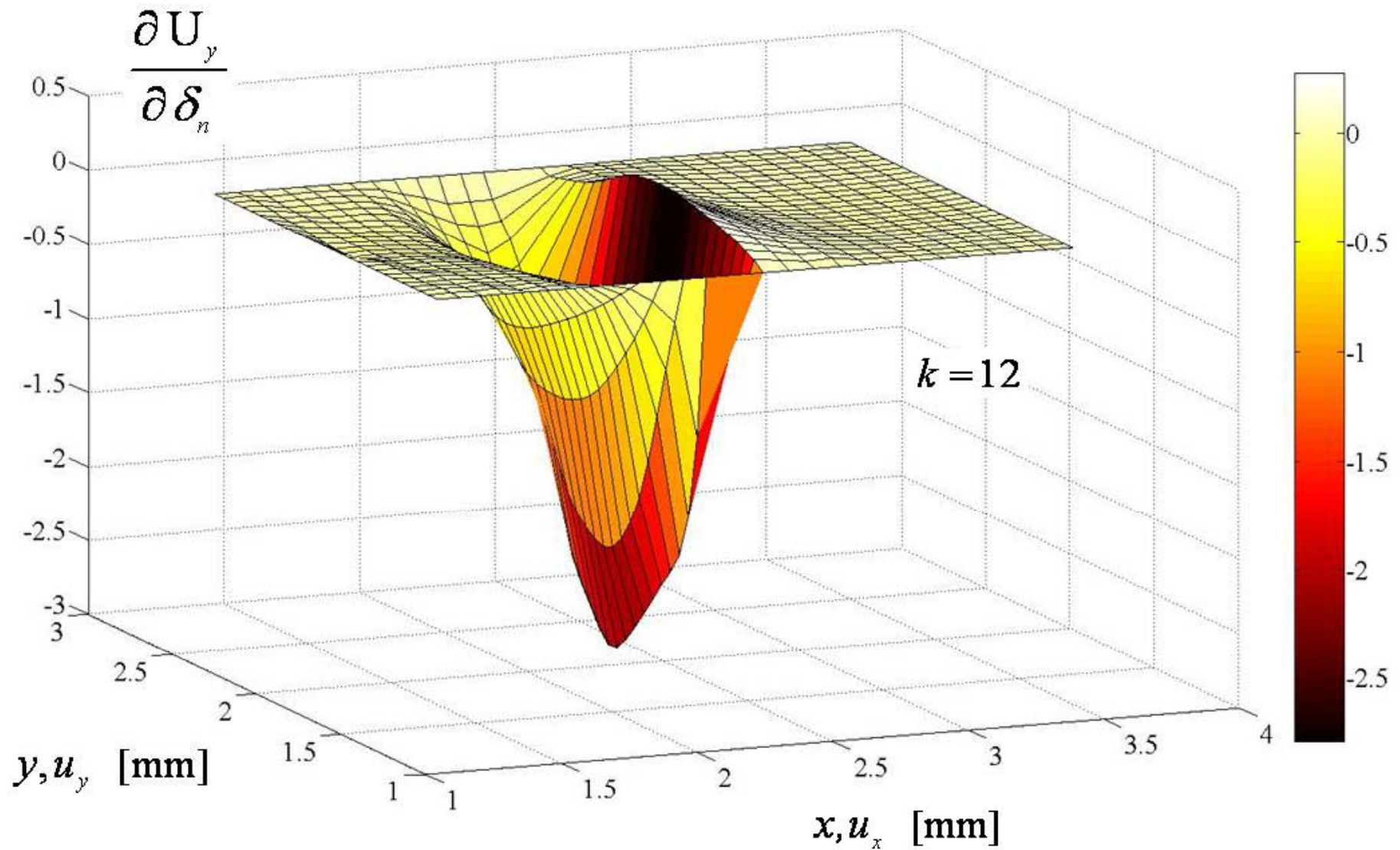
Tangential traction and 95% confidence strip



Normal traction and 95% confidence strip

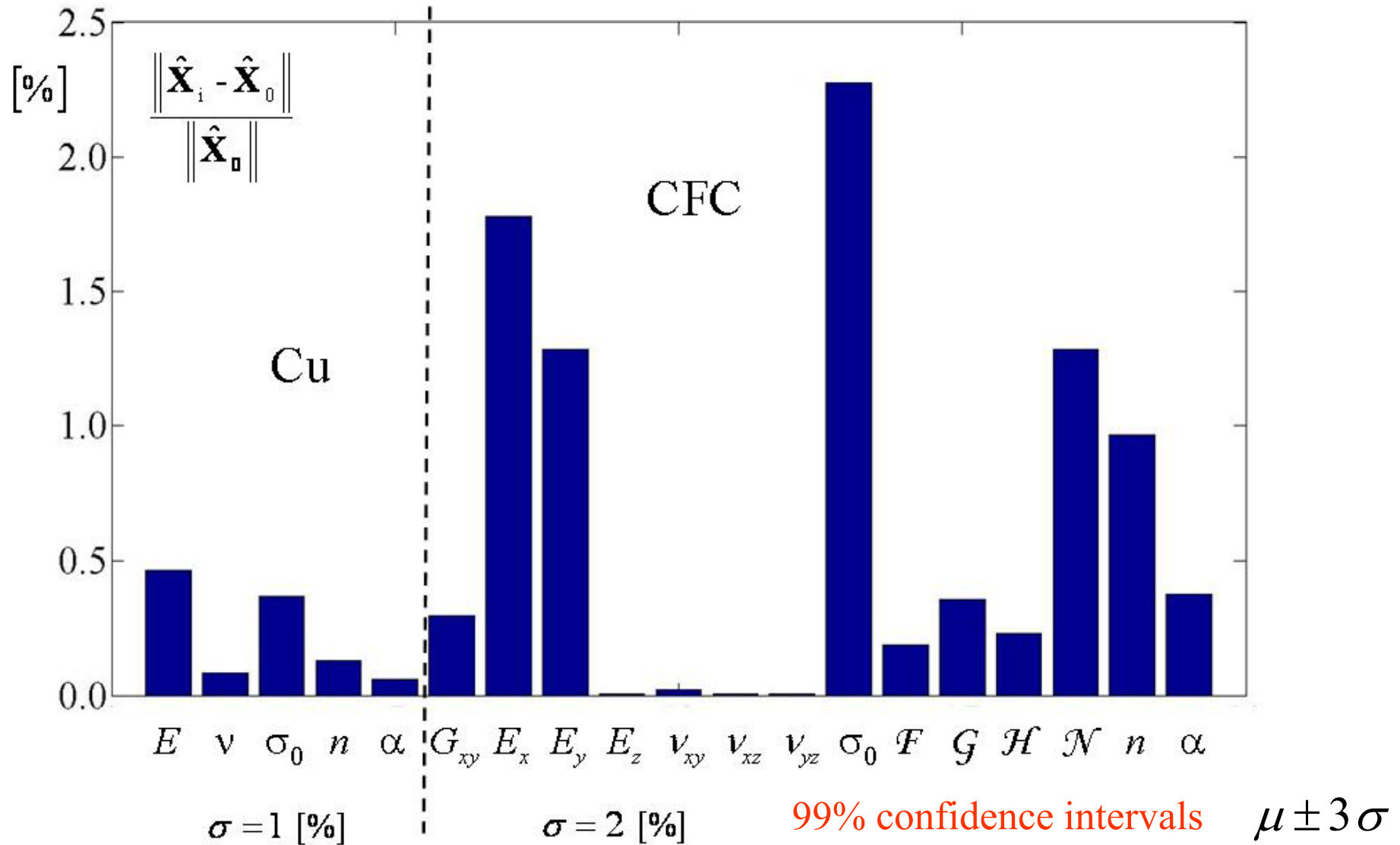


Parameter sensitivity of displacement field



Estimate dependence on a priori information

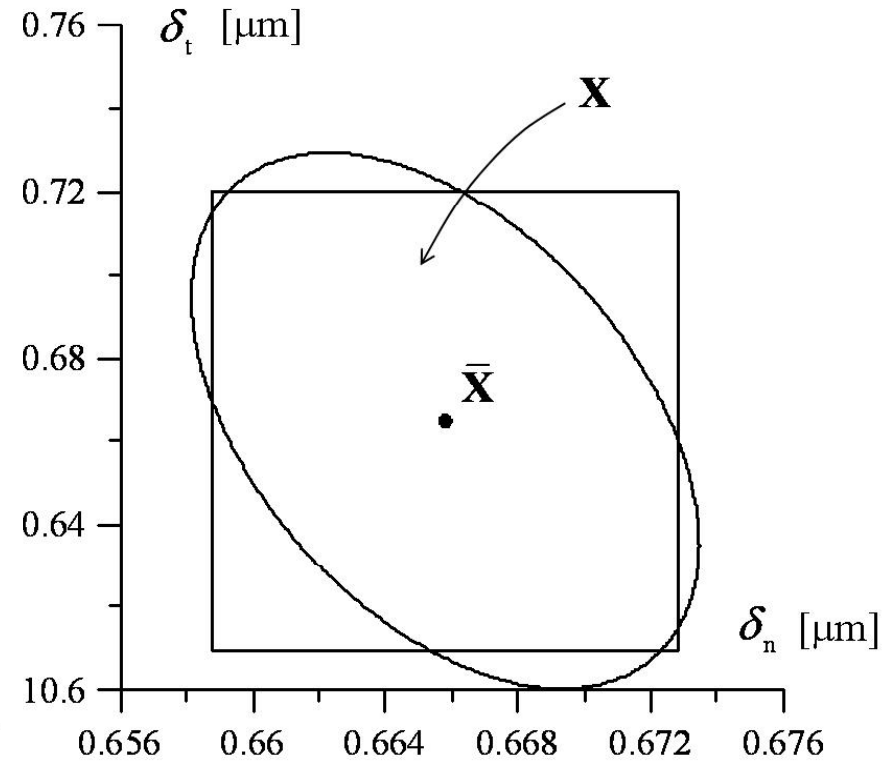
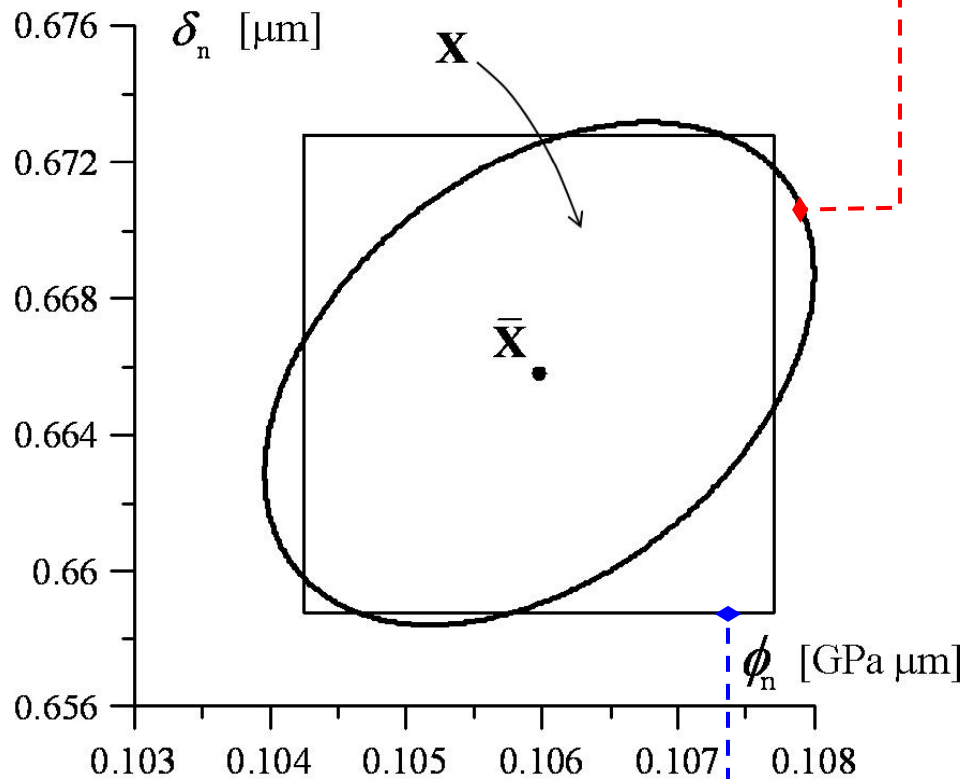
variance assessment by σ -point strategy



Confidence ellipsoids and Bonferroni's domains

$$\begin{aligned} \bar{\phi}_n &= 0.106 \text{ [GPa} \cdot \mu\text{m]} \\ \bar{\delta}_t &= 10.7 \text{ [\mu m]} \quad \bar{\delta}_n = 0.665 \text{ [\mu m]} \end{aligned}$$

$$(\mathbf{X} - \bar{\mathbf{X}})^T \bar{\mathbf{C}}_X^{-1} (\mathbf{X} - \bar{\mathbf{X}}) = \chi_{n_x, 1-\alpha}^2$$



equivalent parallelipedic domains

$$\begin{aligned} \bar{X}_i + z_{\alpha^*} \cdot \sqrt{\bar{C}_{X_{ii}}} \leq X_i \leq \bar{X}_i + z_{1-\alpha^*} \cdot \sqrt{\bar{C}_{X_{ii}}} \\ (i = 1, 2, 3) \quad \alpha^* = \alpha / (2n_x) \end{aligned}$$

Engineering motivations

- ✓ The behavior of masonry structures, strengthened with fiber-reinforced polymer (FRP) thin sheets, is often dominated by delamination of the FRP reinforcement from the support.
- ✓ A further complication is the presence of mortar joints, where cracks may propagate preferentially.
- ✓ This fundamental issue is relatively under-investigated for masonry, especially from a numerical point of view.

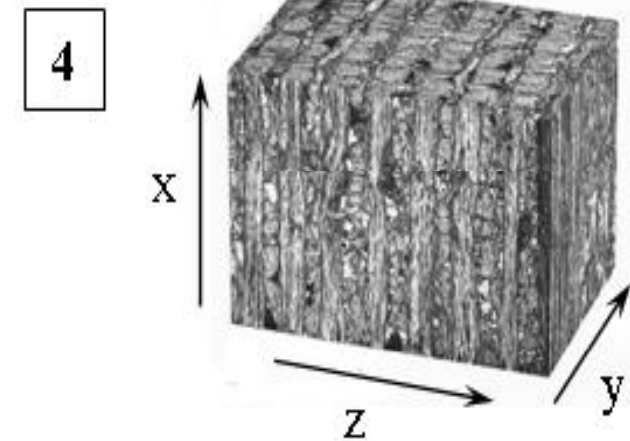
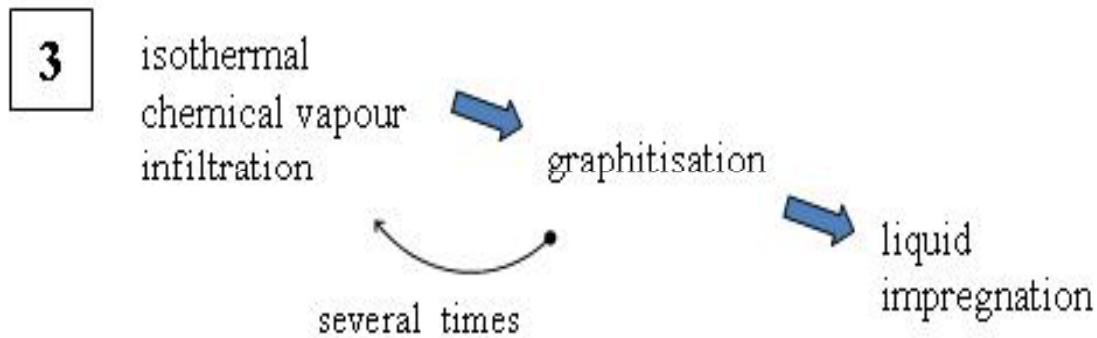
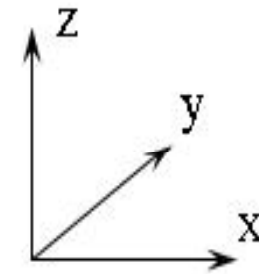
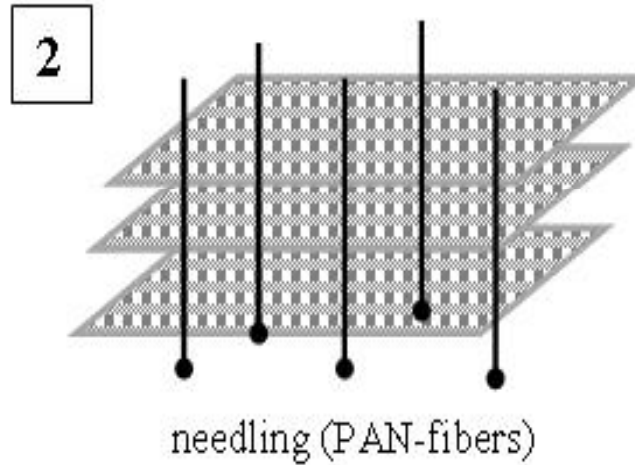
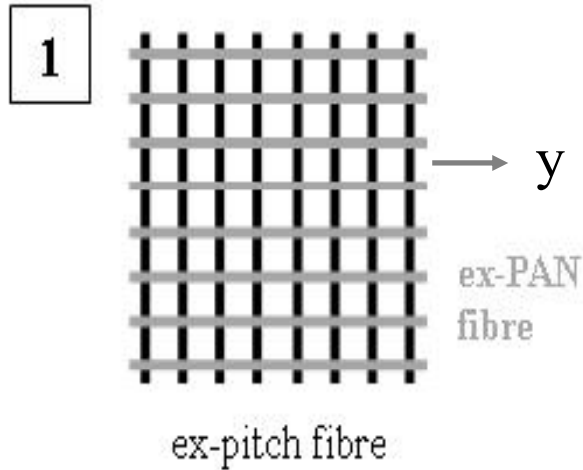
Items to be discussed

- engineering motivations
- shear tests on joined CFC/Cu assemblies
- “optical” inverse problem:
from pictures to displacements
through 2D Digital Image Correlation
- “mechanical” inverse problem:
from full field data to joint properties
through Finite Element Model Updating
- closing remarks and future prospects

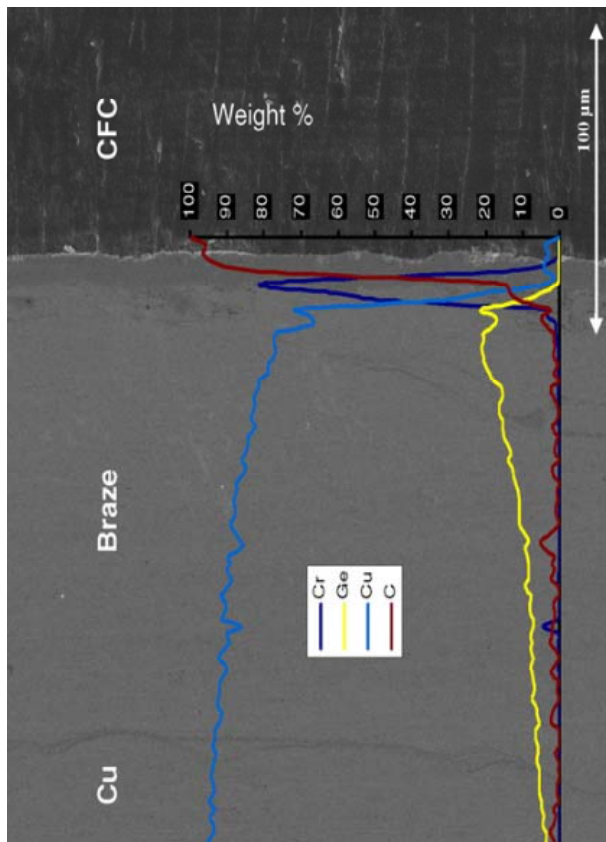
Ceramic Matrix Composite SEP NB31

NOVOLTEX®
preform

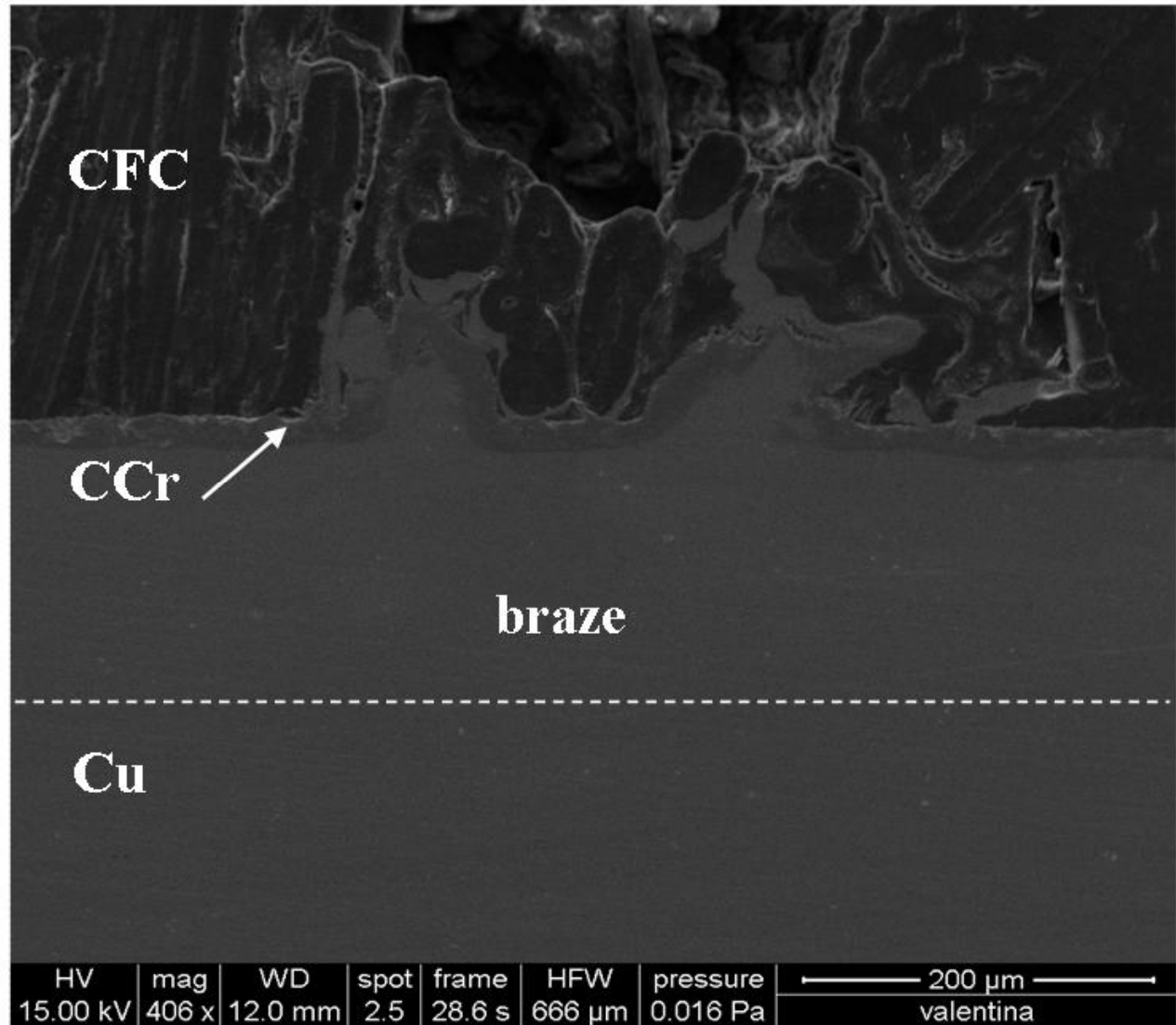
PAN=Poly-
Acrylo-Nitrile



SEM (Philips 525 M)



Energy Dispersive
X-ray spectroscopy



Cromium carbide (about 20 μm thick)
 Cr_7C_3 and Cr_{23}C_6

Adherent nonlinear behavior under plane stress

anisotropic extension of multiaxial Ramberg-Osgood relationship

Mücke & Bernhardt
CMAME (2003)

3 parameters

$$\boldsymbol{\varepsilon} = \boldsymbol{\varepsilon}_{\text{el}} + \boldsymbol{\varepsilon}_{\text{pl}} = \mathbf{C} \boldsymbol{\sigma} + \frac{\alpha}{E_R} \left[\frac{\sigma_{\text{eq}}^{n-1}}{\sigma_0^{n-1}} \right] \mathcal{M} \mathbf{s}$$

deviatoric stress tensor

$$\mathbf{s} \equiv \boldsymbol{\sigma} - (\text{tr } \boldsymbol{\sigma} / 3) \mathbf{1}$$

$$\sigma_{\text{eq}} \equiv \left(\mathbf{s}^T \mathcal{M} \mathbf{s} \right)^{1/2} \text{ equivalent stress}$$

$$\mathbf{C} \equiv \begin{pmatrix} 1/E_x & -\nu_{yx}/E_y & -\nu_{zx}/E_z & 0 \\ -\nu_{xy}/E_x & 1/E_y & -\nu_{zy}/E_z & 0 \\ -\nu_{xz}/E_x & -\nu_{yz}/E_y & 1/E_z & 0 \\ 0 & 0 & 0 & 1/G_{xy} \end{pmatrix}$$

$$\mathcal{M} \equiv \begin{pmatrix} \mathcal{G} + \mathcal{H} & -\mathcal{H} & -\mathcal{G} & 0 \\ -\mathcal{H} & \mathcal{H} + \mathcal{F} & -\mathcal{F} & 0 \\ -\mathcal{G} & -\mathcal{F} & \mathcal{F} + \mathcal{G} & 0 \\ 0 & 0 & 0 & 2\mathcal{N} \end{pmatrix}$$

$\sum \mathcal{M}([1:3], [1:3]) = 0$

3 parameters

incompressible strain $\text{tr}(\boldsymbol{\varepsilon}_{\text{pl}}) = 0$

$\nu_{ij}/E_i = \nu_{ji}/E_j \quad i, j = x, y, z$

7 parameters

for isotropic Cu $F = G = H = \frac{1}{2}; \mathcal{N} = \frac{3}{2}$

First-order sensitivity analysis by Direct Differentiation Method

FE equilibrium eqs
at the free dofs
at step n

$$\mathbf{F}_{\text{int}}(\mathbf{X}; \mathbf{U}^n(\mathbf{X}); \bar{\mathbf{U}}_{\partial\Omega}^n) = \mathbf{0}$$

Here dependence only upon \mathbf{U}^n

unknown parameters

prescribed displacements
along the boundary

$$\frac{\partial}{\partial \mathbf{X}^T} \mathbf{F}_{\text{int}} = \left(\frac{\partial \mathbf{F}_{\text{int}}}{\partial \mathbf{U}^n} \right) \frac{\partial \mathbf{U}^n}{\partial \mathbf{X}^T} + \frac{\partial \mathbf{F}_{\text{int}}}{\partial \mathbf{X}^T} = \mathbf{0}$$

pseudo-load vector

$$\mathbf{K}_{\text{tan}} \frac{\partial \mathbf{U}^n}{\partial \mathbf{X}^T} = \left(- \frac{\partial \mathbf{F}_{\text{int}}}{\partial \mathbf{X}^T} \right)$$

assembled tangent stiffness matrix,
already available in a Newton scheme