

*Pre-Conference Workshop: New approaches to address pavement failure more realistically in asphaltic pavement design methods*

## Design and management approaches addressing asphalt pavement performance

Michael P. Wistuba

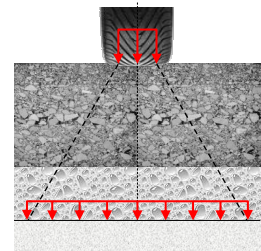
Braunschweig Pavement Engineering Centre  
Technische Universität Braunschweig

### Background



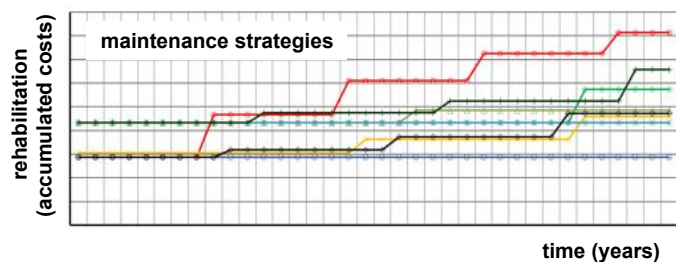
### Predicting asphalt pavement performance

- Key element in pavement design
  - minimum layer thicknesses to support loading in function of material properties
  - ranking of materials
  - estimation of residual value

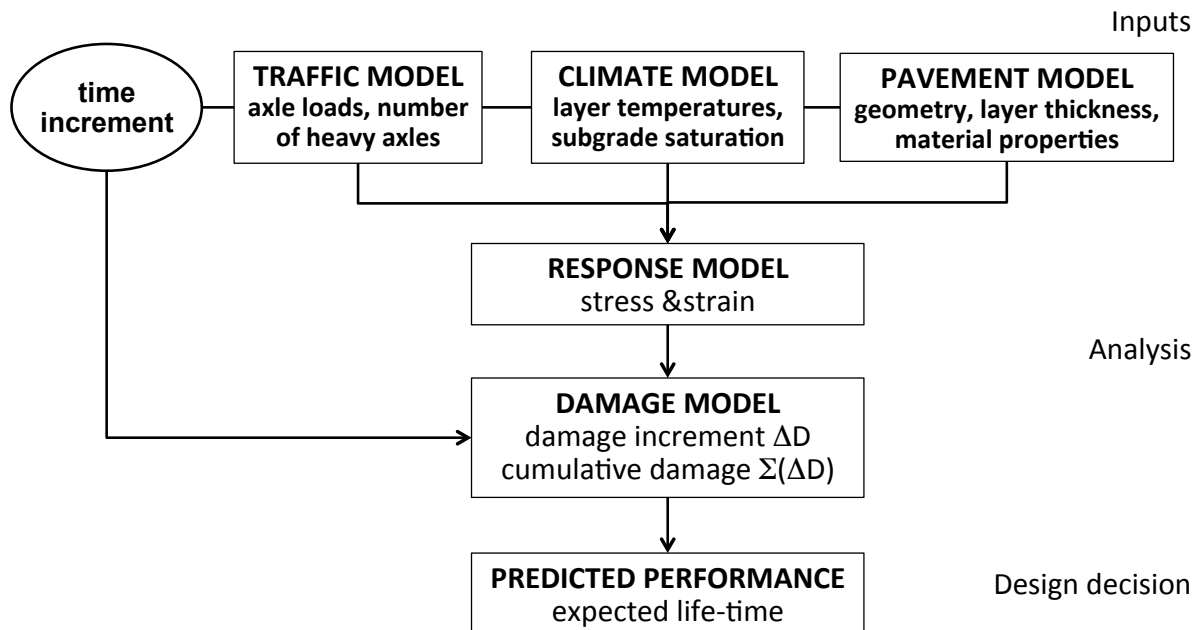


### • Key element in pavement management

- life-cycle-analysis
  - residual value
  - maintenance intervals
  - budgetary needs



## (a) Mechanical Empirical Pavement Design (MEPD)



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## (a) Mechanical Empirical Pavement Design (MEPD)

- German pavement design guide

*RDO Asphalt 09 "Richtlinien für die rechnerische Dimensionierung des Oberbaus von Verkehrsflächen mit Asphaltdeckschicht"*

- basic, traditional pavement design approach as in many other countries



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## Multilayer Elastic Theory

### layered elastic model

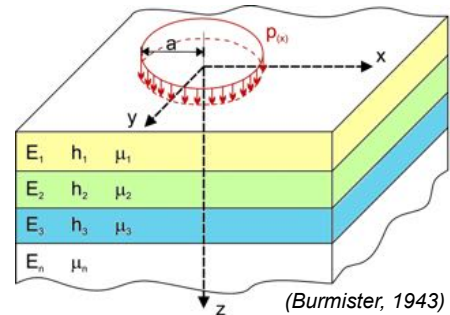
- homogeneous, isotropic, linear elastic
- infinite horizontal layer extension
- infinite vertical subgrade extension
- stick or slip layer interfaces

### limited number of input data

- layer thicknesses
- material properties (elastic modulus, Poisson ratio, layer friction)
- force (magnitude of wheel load) and load geometry (tire patch load)

### materials are not stressed beyond their elastic ranges

- suitable for short-term loading at low to moderate (?) temperatures



# German pavement design guide

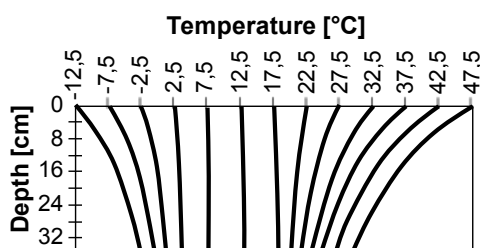
### simple model applicable for high numbers of load repetitions

### E. g., German design guide:

11 load classes for consideration of truck traffic

13 characteristic temperature distributions

= **143 loading cases** for design analysis (advantage: short calculation time)



$$\sigma_r = \frac{E}{(1+\nu) \cdot (1-2 \cdot \nu)} \cdot \left\{ (1-\nu) \cdot \frac{\delta u}{\delta r} + \nu \cdot \left( \frac{u}{r} + \frac{\delta w}{\delta z} \right) \right\}$$

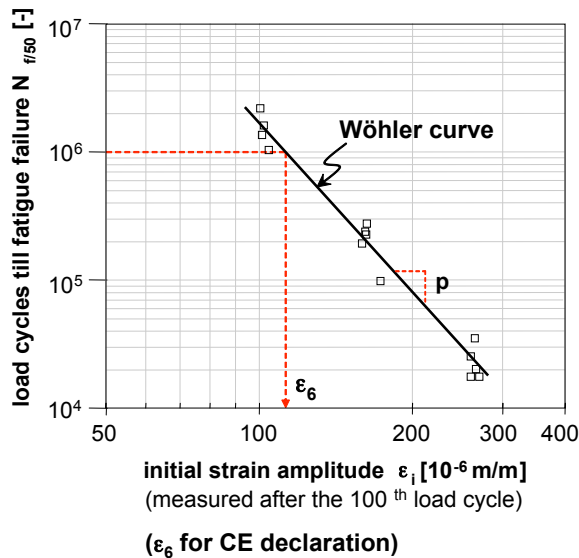
$$\sigma_t = \frac{E}{(1+\nu) \cdot (1-2 \cdot \nu)} \cdot \left\{ (1-\nu) \cdot \frac{u}{r} + \nu \cdot \left( \frac{u}{r} + \frac{\delta w}{\delta z} \right) \right\}$$

$$\sigma_z = \frac{E}{(1+\nu) \cdot (1-2 \cdot \nu)} \cdot \left\{ (1-\nu) \cdot \frac{\delta w}{\delta z} + \nu \cdot \left( \frac{u}{r} + \frac{\delta u}{\delta r} \right) \right\}$$

$$\tau_{rz} = \frac{E}{2 \cdot (1+\nu)} \cdot \left\{ \frac{\delta u}{\delta z} + \frac{\delta w}{\delta r} \right\}$$

## Dominant deterioration mechanism considered

- **material fatigue** is crucial design criterion to avoid structural failure
- **Wöhler curves** are derived from laboratory fatigue testing
- linear summation of the damaging effects of individual loads (**Miner**)



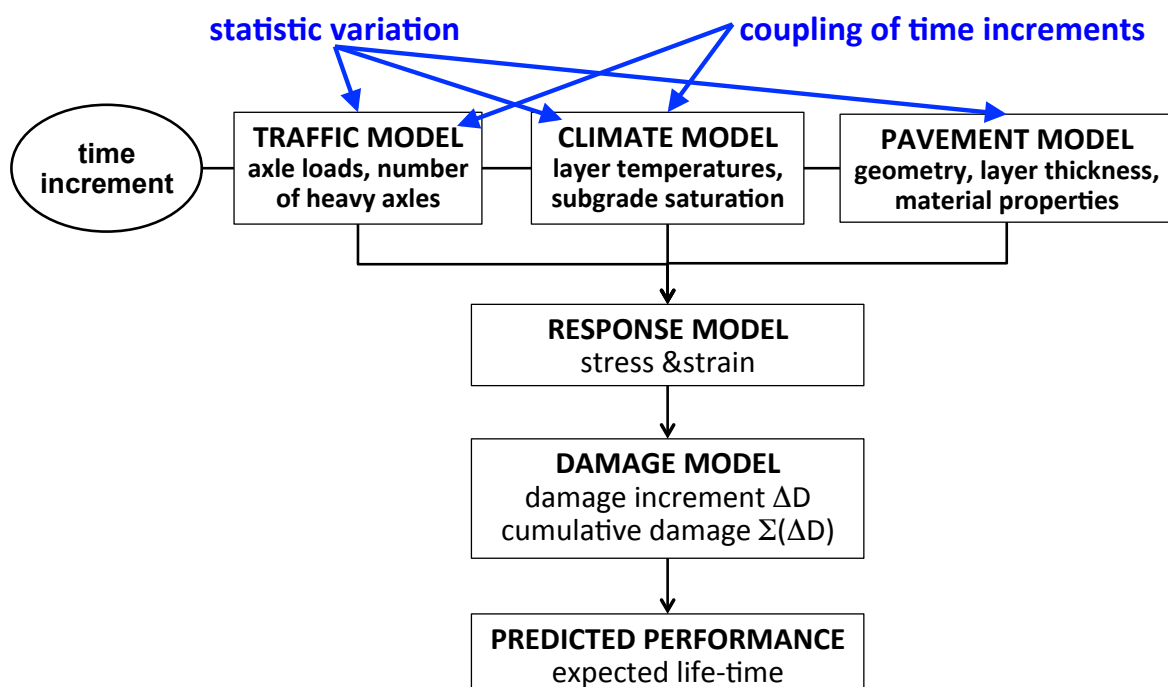
**Fatigue law:**

$$N_f = k_1 \cdot \left(\frac{I}{\epsilon_0}\right)^{k_2}$$

**Miner rule** (linear accumulation of incremental damage):

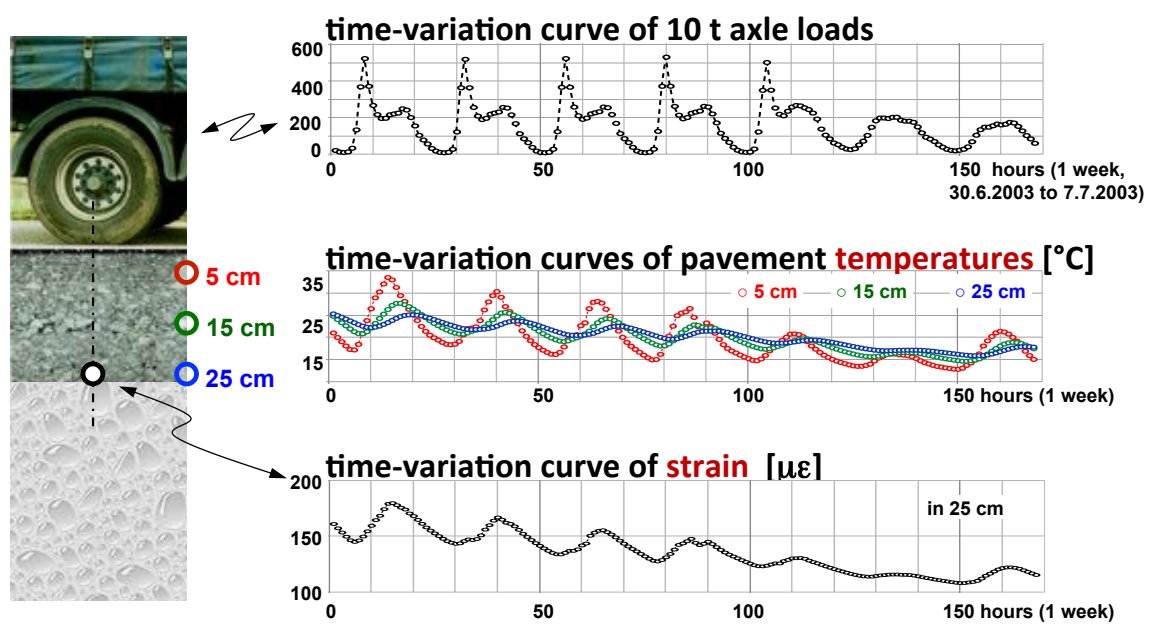
$$C = \sum \left( \frac{N_{\text{vorh}}}{N_{\text{zul}}} \cdot p \right) \leq 1$$

## Predicting asphalt pavement performance



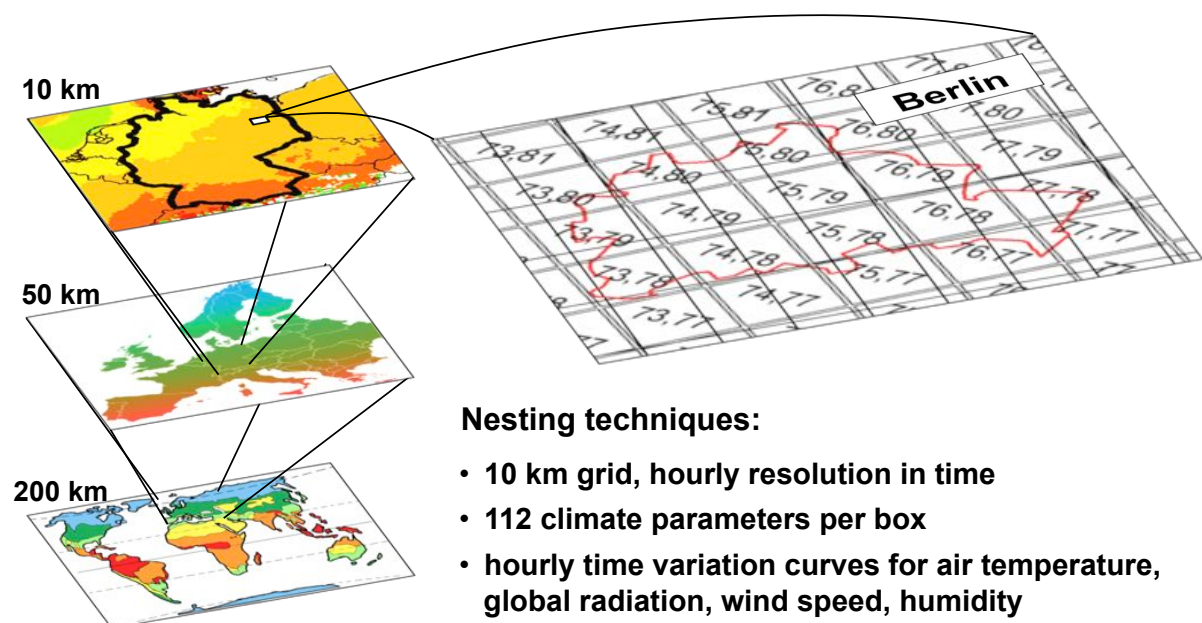
# Stress analysis in time increments

- time-accurate superposition of traffic & temperature



→ superposition and **analysis of damage** in time increments

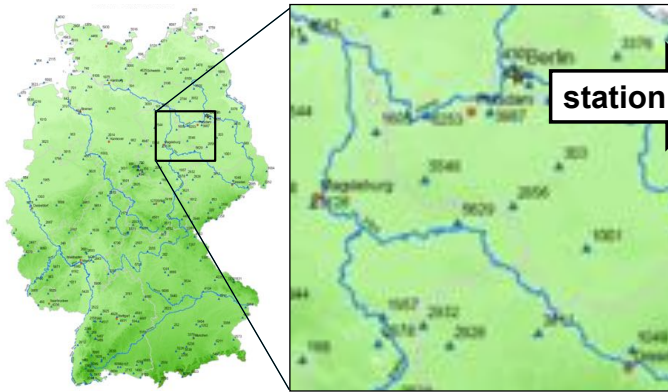
# Regional climate model (REMO)



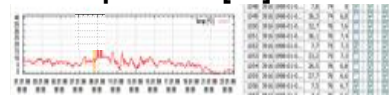
# Regional climate model (REMO)

## Climate Stations

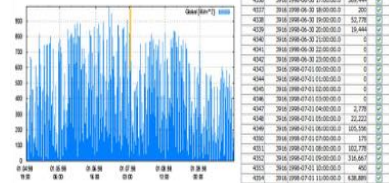
(DWD, 2012)



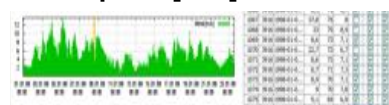
### air temperature [°C]



### global radiation [W/cm<sup>2</sup>]



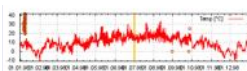
### wind speed [m/s]



### humidity [%]



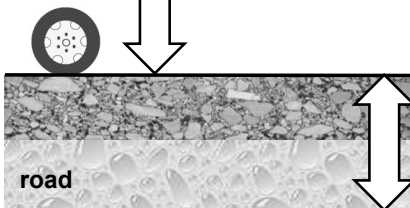
# Pavement temperature distribution



**climate data:** meteorological observations at climate station

**heat transfer at pavement surface**  
energy balance law

$$Q_{\text{net}} = (1 - \alpha) G + \epsilon_a F T_{\text{air}}^4 - \epsilon_b F T_{\text{surf}}^4 - \lambda \frac{\Delta T}{d} + \alpha_K (T_{\text{air}} - T_{\text{surf}}) = 0$$



**heat flux within pavement layers**

**Fourier heat balance law**

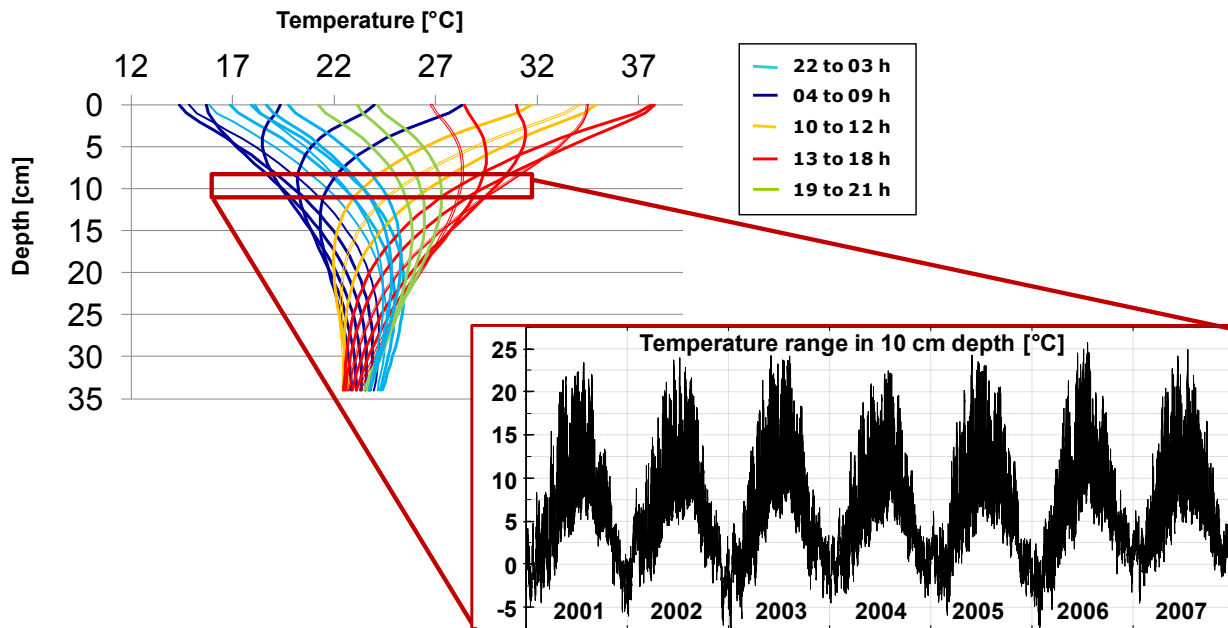
$$C \frac{\partial T}{\partial t} = -\text{div } q \quad T(x, t=0) = T_0(x)$$

$$q = -k \text{ grad } T \quad q_n = \alpha (T - T_\infty)$$

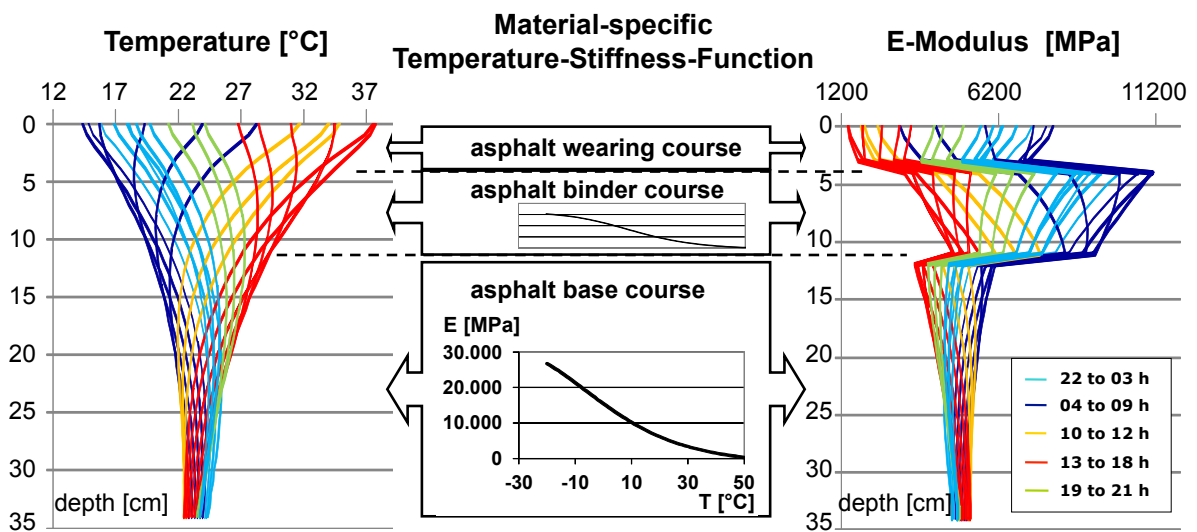
C ... vol. heat capacity  
q ... heat flux vector  
k ... heat conduction coeff.  
α ... heat transfer coeff.  
T<sub>∞</sub> ... ambient temperature

[Krebs & Böllinger, 1981; Wistuba, 2002; Villaret et al., 2007]

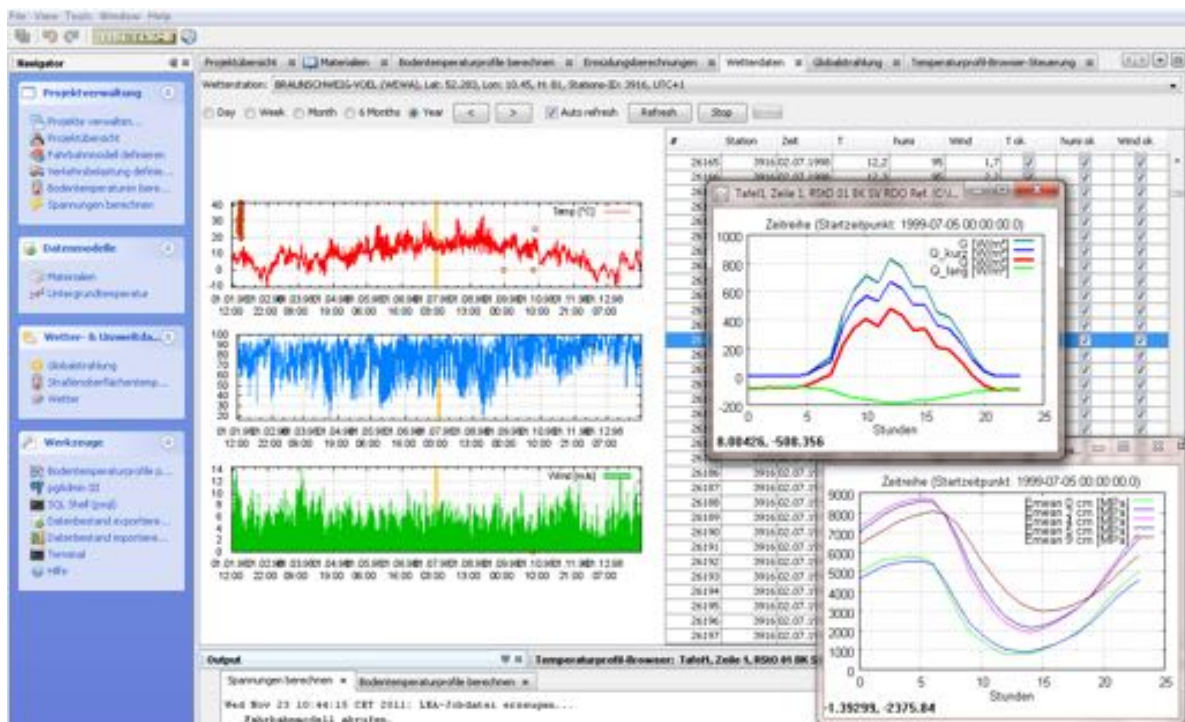
# Pavement temperature distribution



# Pavement temperature distribution

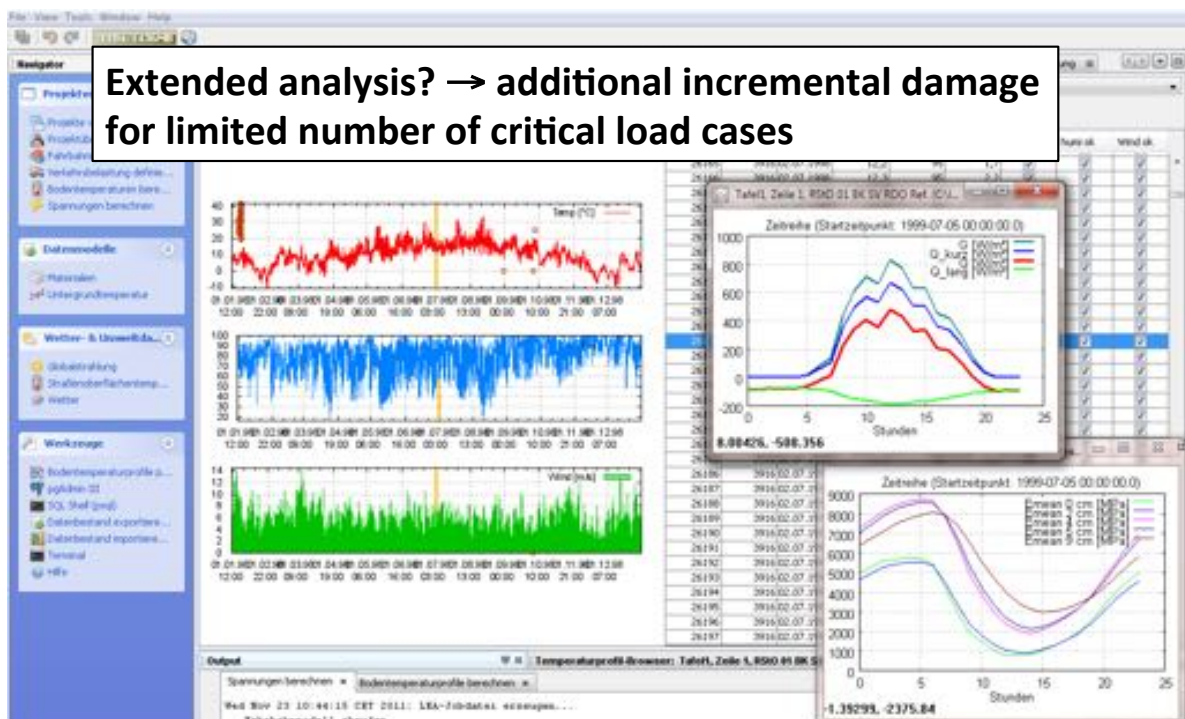


# Software for incremental stress analysis



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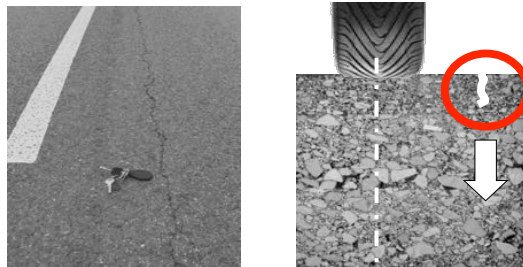
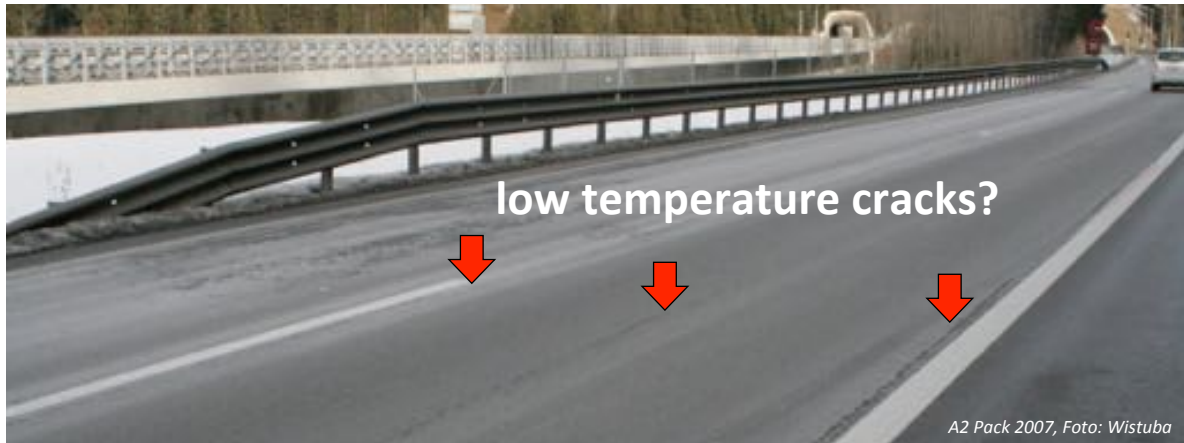
# Software for incremental stress analysis



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# Longitudinal low temperature top down cracking

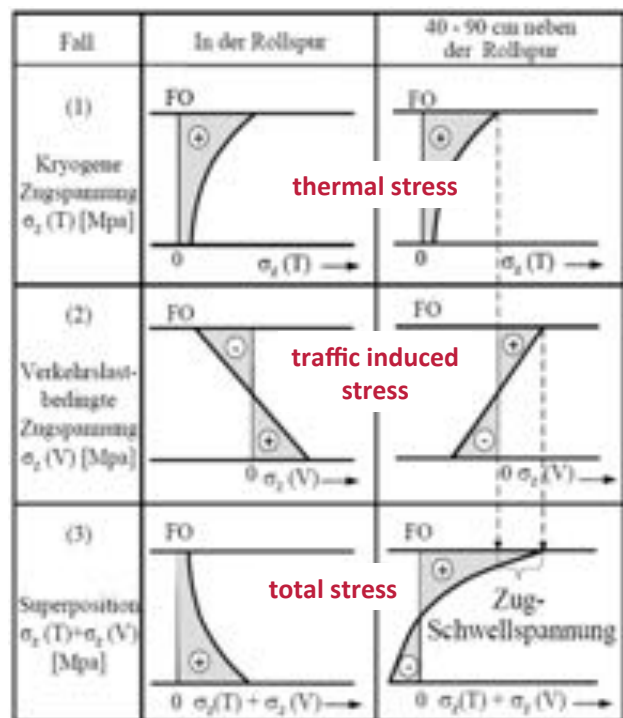
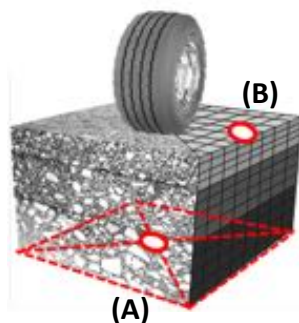


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# Longitudinal low temperature top down cracking

(A) in wheel path (B) beside wheel path

- Question: can low-temp. stress be explained from LE calculation?
  - due to a negative temperature gradient (drop in temperature) and
  - low stress relaxation at low temperature

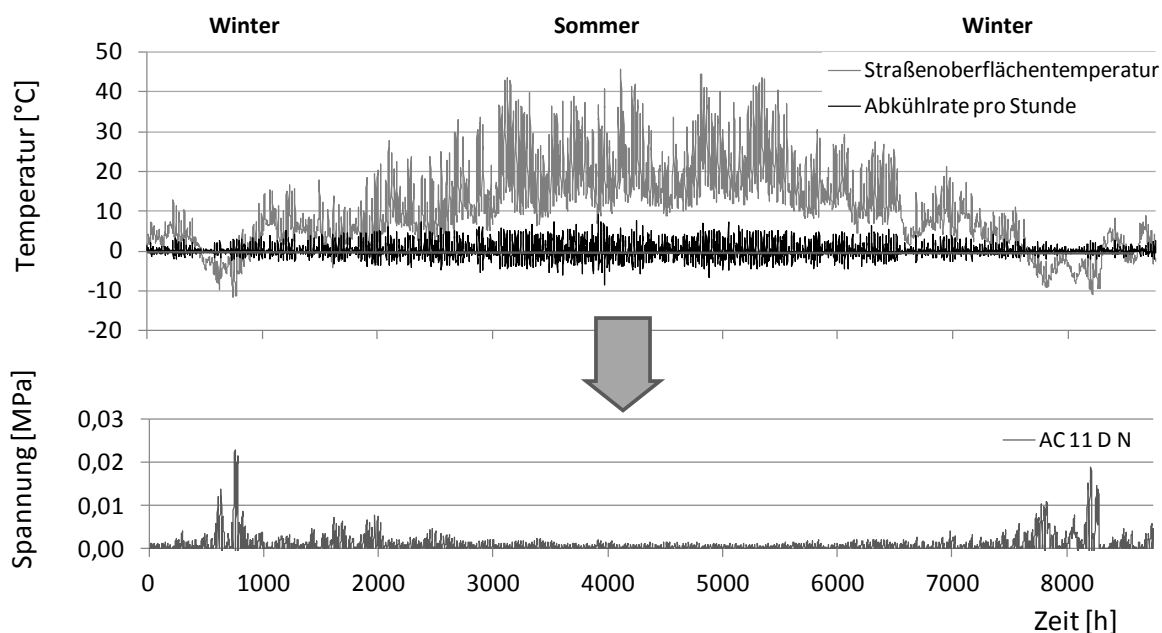


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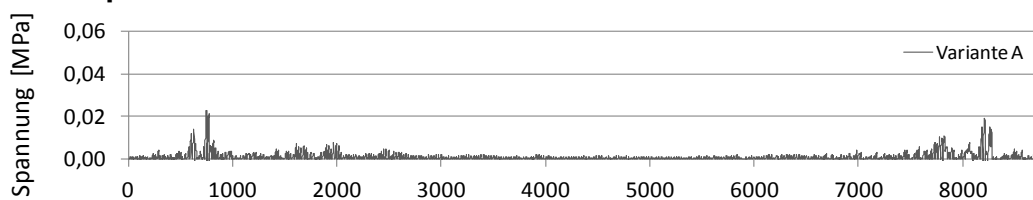
# Longitudinal low temperature top down cracking

## Thermal stress at surface 1 m beside wheel track

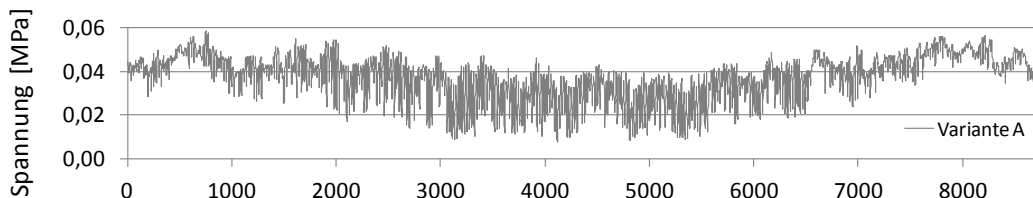


# Longitudinal low temperature top down cracking

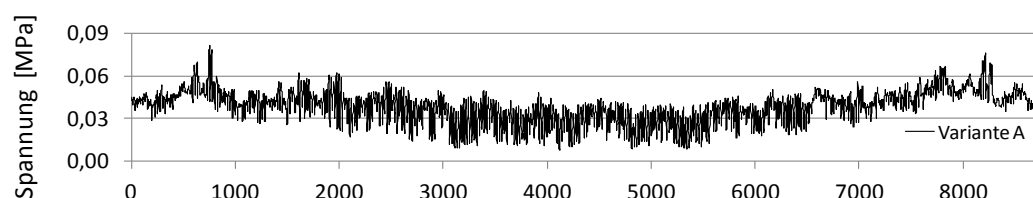
## Low temperature stress at surface



## Traffic induced stress at surface

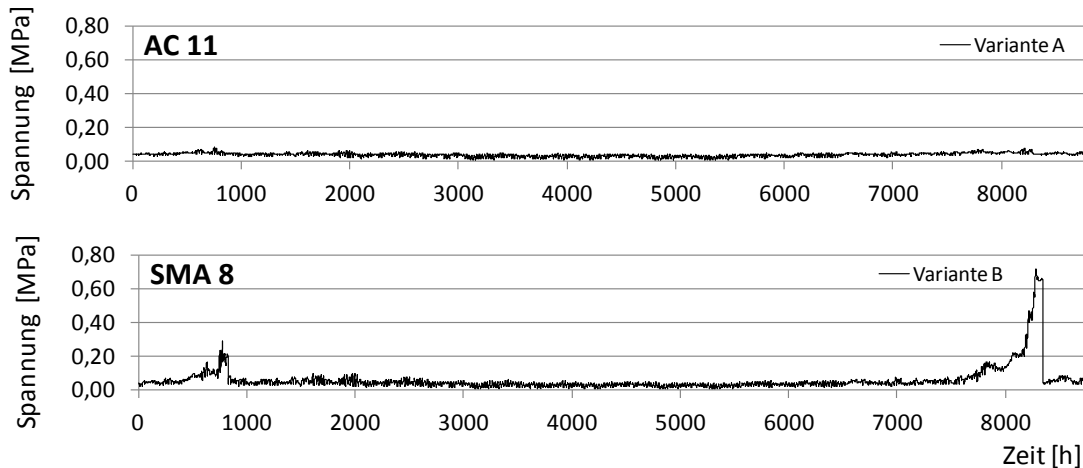


## Total stress at surface



## Longitudinal low temperature top down cracking

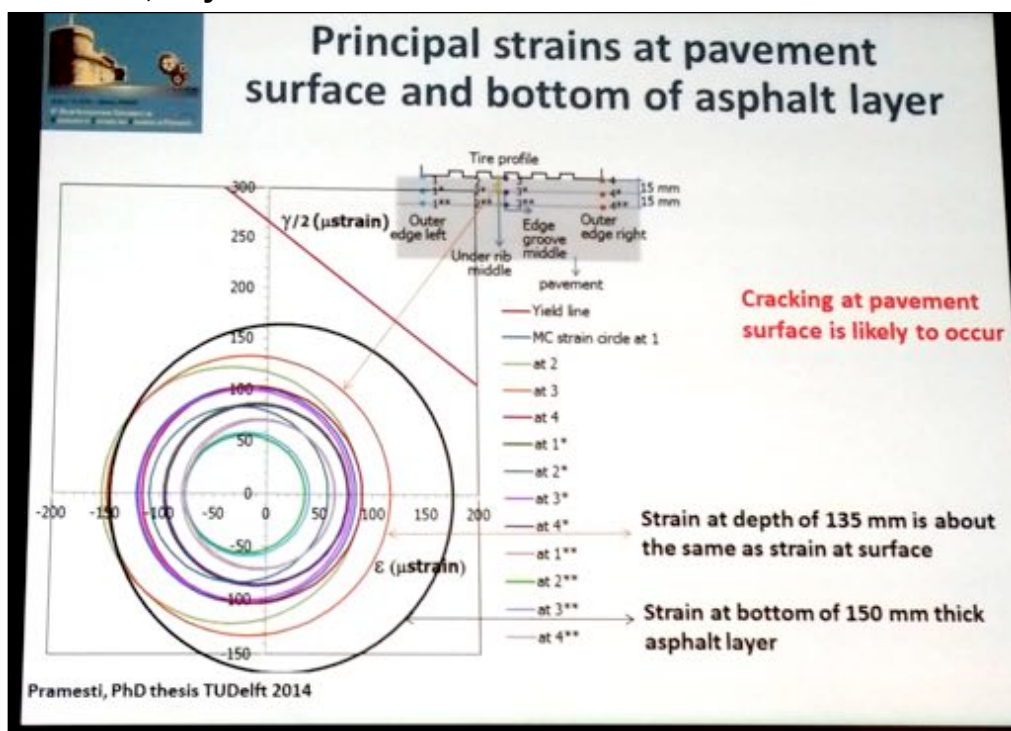
- can not be explained from LE calculation of low-temperature stress (only in very rare cases and for extremely hard bitumens). Generally, this type of crack is supposed to be driven by other crack mechanisms (shear at moderate and high temperatures?)



Walther, A. & Wistuba, M. 2012. Mechanistic Pavement Design Considering Bottom-Up And Top-Down Cracking. *Proc., 7<sup>th</sup> RILEM Int. Conf. On Cracking in Pavements, 20-22 June, 2012, Delft.*


## Predicting asphalt pavement performance

Molenaar, Keynote MCD 2016 Nantes



# Predicting asphalt pavement performance

Molenaar, Keynote MCD 2016 Nantes



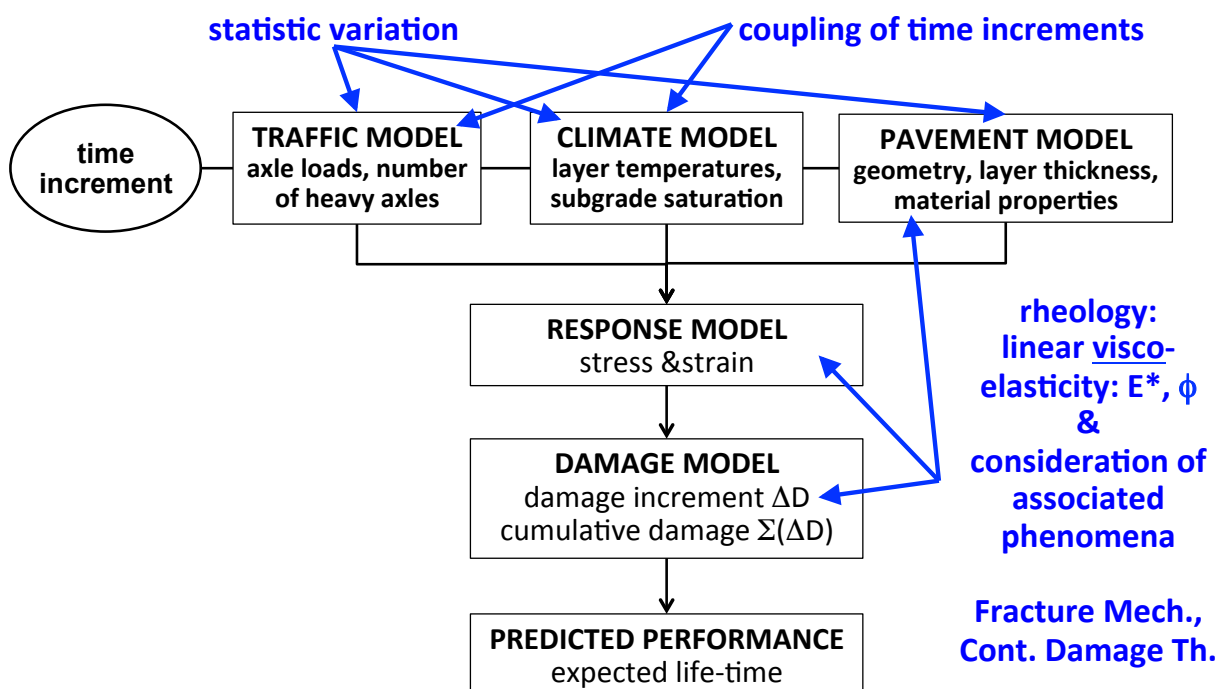
## Conclusion on top down cracking

- Complex contact pressure distributions with high peak stresses will result in high tensile strains at pavement surface
- Surface/top down cracking is likely to occur because of these high tensile strains
- Top down cracking will be dominant in thicker asphalt pavements
- Hardening of surface layer will aggravate problem
- Durable, high fatigue and permanent deformation resistant mixtures will solve much of the problem

8<sup>th</sup> RILEM International Conference on Mechanisms of Cracking and Debonding in Pavements (MCD2016) 36

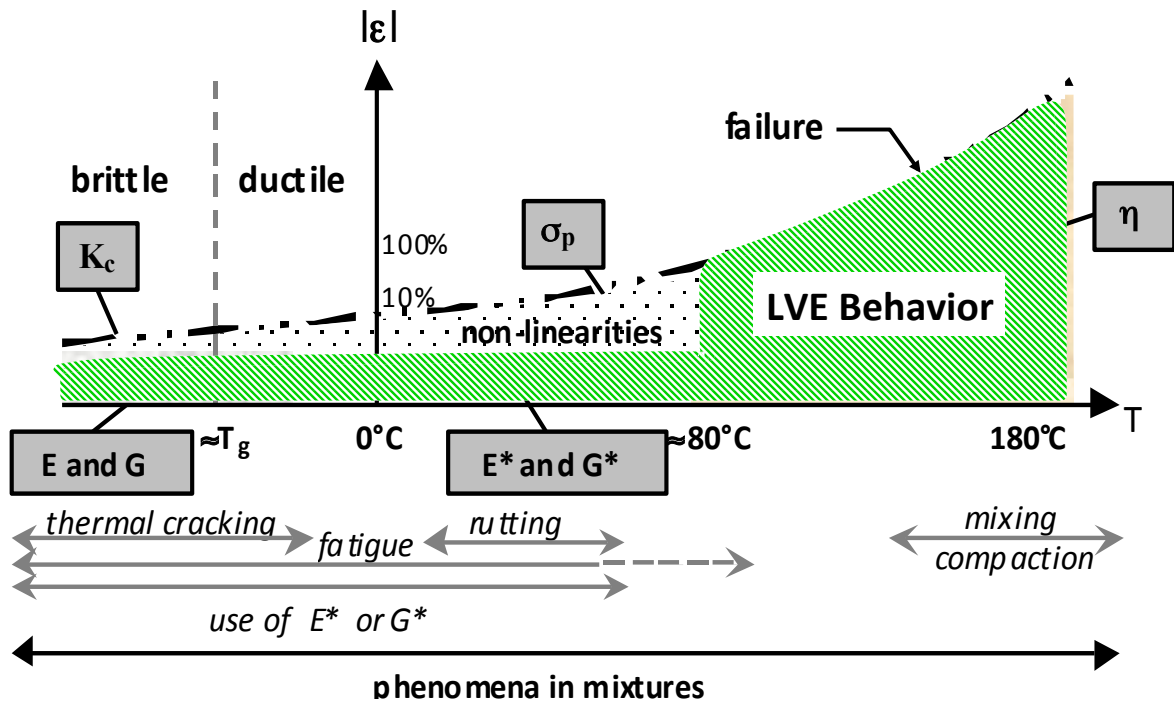
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# Predicting asphalt pavement performance

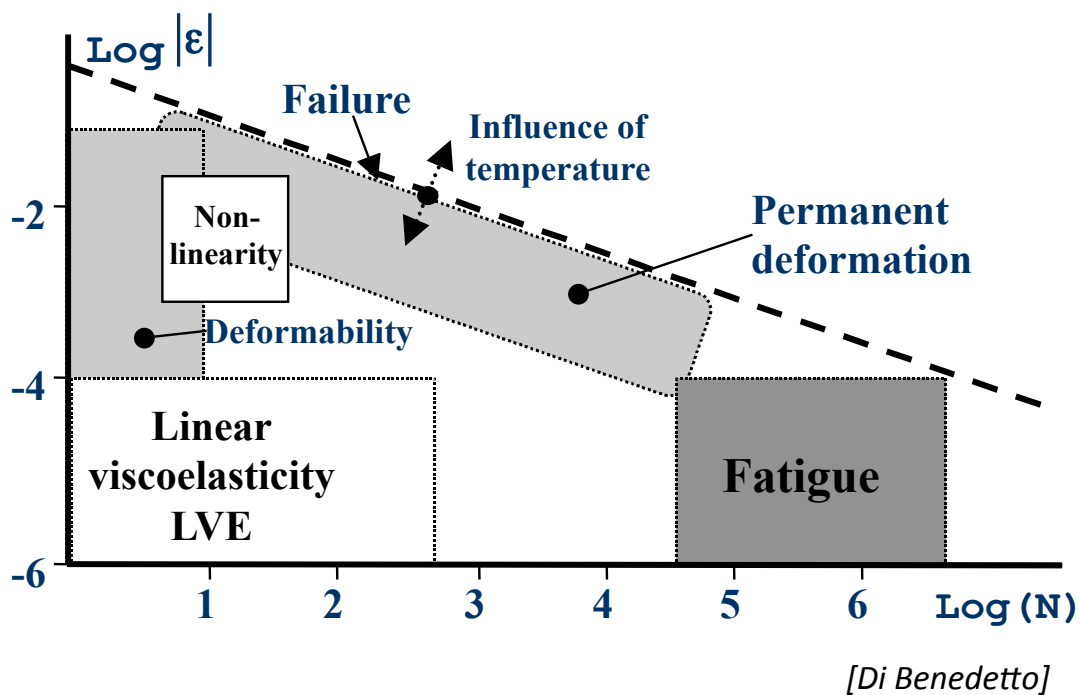


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# Linear visco-elastic bitumen behavior

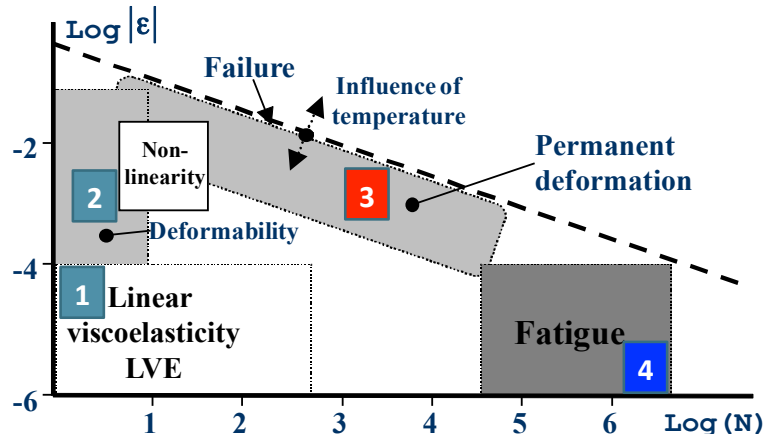


# Domains of asphalt mix behaviour



## Associated phenomena

- Linear viscoelasticity
- Non linearity
- Fatigue
- Healing
- Thixotropy
- Crack propagation
- Permanent deformation
- Brittle failure
- Viscoplastic flow
- Thermo-mechanical coupling
- transfer to 3D



[Di Benedetto]

## Fracture mechanics

- Low temperature cracking represents a serious distress for asphalt pavements in cold regions. **Crack failure properties** of asphalt mixture are crucial for design.
  - They are used as input in the Thermal Cracking model which is part of the current **Mechanistic Empirical Pavement Design Guide acc. to AASHTO 2008**.
  - **Fracture strength** properties of asphalt mixture can be successfully predicted **from SCB fracture testing** without performing IDT tests at low temperature.
- See **paper N° 66860** (Cannone Falchetto, Moon & Wistuba: Numerical correlation between low temperature SCB fracture and IDT strength of asphalt mixture using FEM analysis)

# Continuum Damage Theory

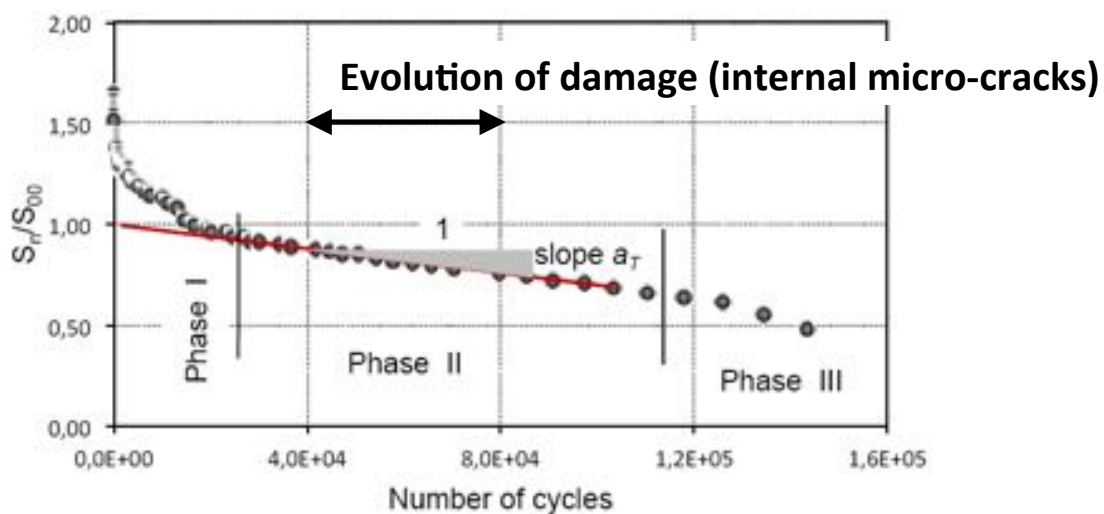
- Characteristic curve representing the evolution of internal damage  $D$  in the mixture in function of pseudo strain energy  $W^R$  and of a material property  $\alpha$

$$\dot{D} = \left( -\frac{\partial W^R}{\partial D} \right)^\alpha$$

$$N_f = k_1 \cdot \left( \frac{1}{\varepsilon_0} \right)^{k_2}$$

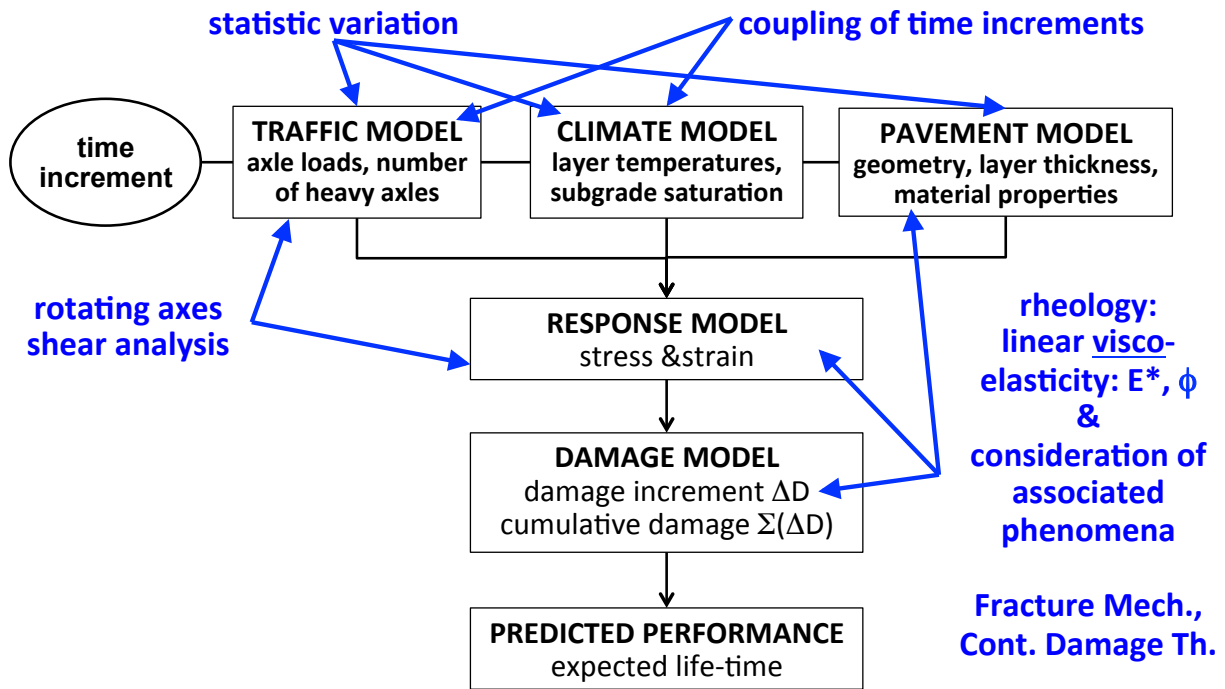
$k_2 = 2 \alpha$  (Lee et al., 2003;  
Kim et al. 2006)

# Continuum Damage Theory



[Di Benedetto]

# Predicting asphalt pavement performance



# Predicting asphalt pavement performance

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## Fatigue life based on maximum tensile strain?

We take into account the major principal (tensile) strain. What are we doing with the intermediate and minor principal strain? Are we ignoring them?

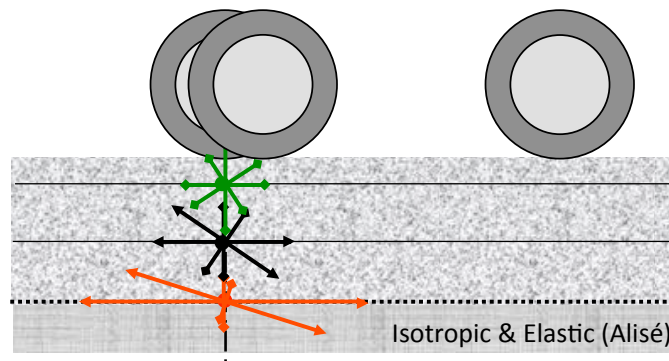
The diagram shows a 3D cube with three principal strains:  $\epsilon_3 = -161 \mu\text{m/m}$  (vertical),  $\epsilon_2 = 111 \mu\text{m/m}$  (diagonal), and  $\epsilon_1 = 164 \mu\text{m/m}$  (horizontal). To the right, a Mohr's circle is shown with values -161, 111, and 164. The text 'Used for design' is below the circle.

200 mm 800 kPa  
150 mm asphalt 5000 MPa  
300 mm base 250 MPa  
Subgrade 100 MPa

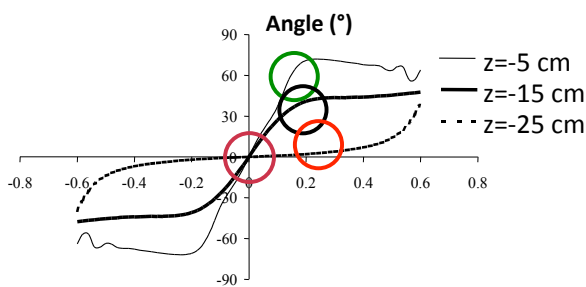
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# Rotating axles: stress paths in layers



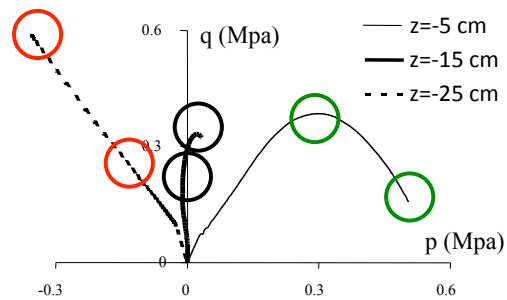
## Rotation of axes



Distance from the center of the wheel (m)

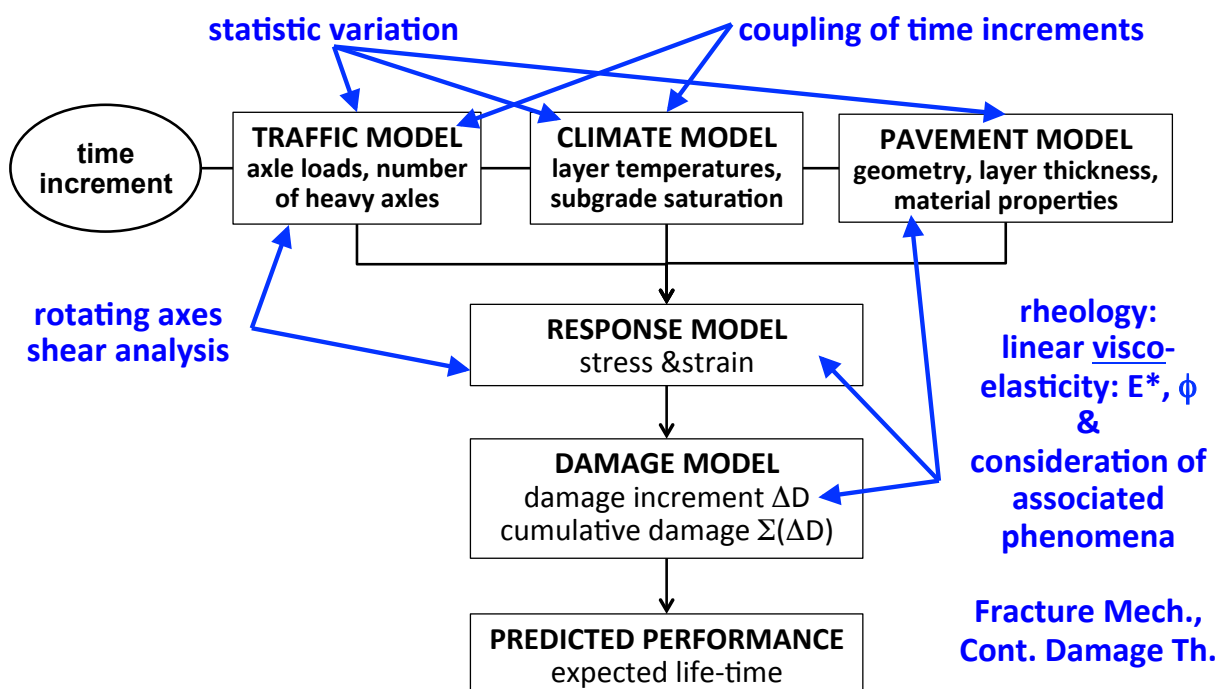
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## Principal stress



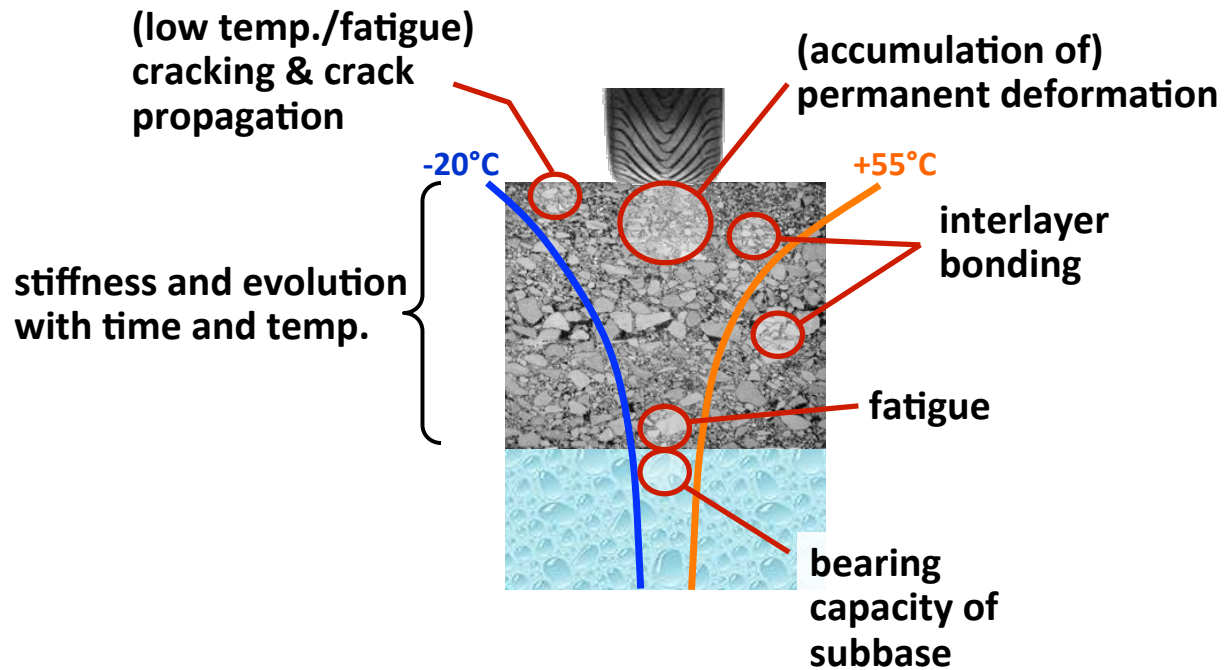
[Di Benedetto]

# Predicting asphalt pavement performance



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# Key issues in pavement design



# Predicting asphalt pavement performance

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**Pavement Design**

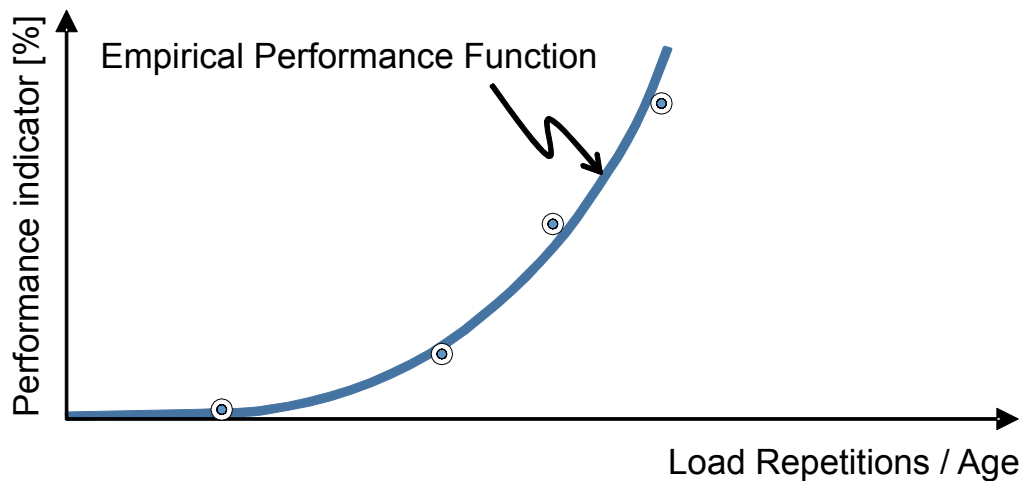
Use of advanced models and “complex” material tests is still far away from day to day practice

Correlating “complexity” to “simplicity” is therefore important

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## (b) Pavement management: Empirical Perform. Function

Pavement condition parameters are measured in time intervals, and a time-correlation function is derived describing the temporal change of the condition parameter



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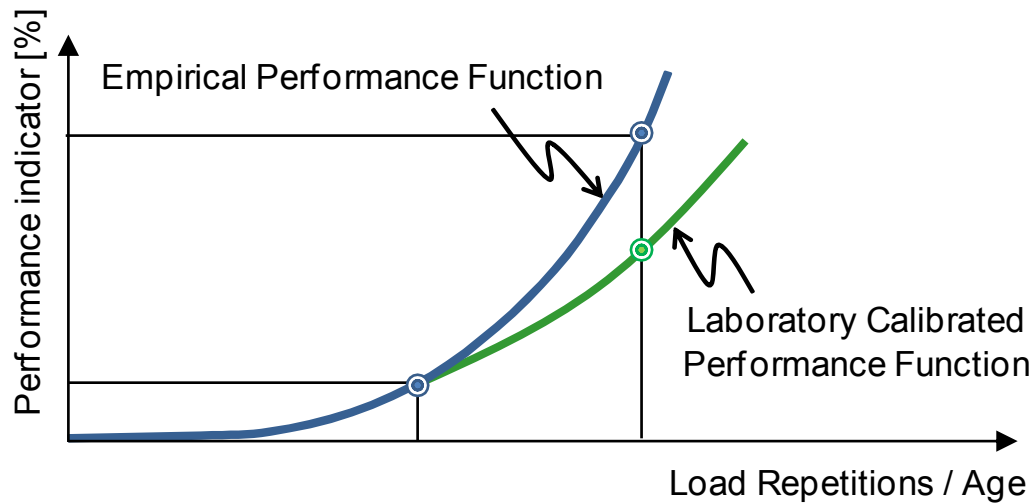
## Empirical Performance Function (EPF)

- EPF do not incorporate structural parameters but are based on **surface characteristics** / defects only
  - EPF do not work for innovative materials and structures (new products, recycled materials)
- few effort has been made to incorporate a material science based approach for performance prediction in the frame of PMS
- need for **analysis of pavement condition in function of incremental distress mechanism** → linked with MEPD approach

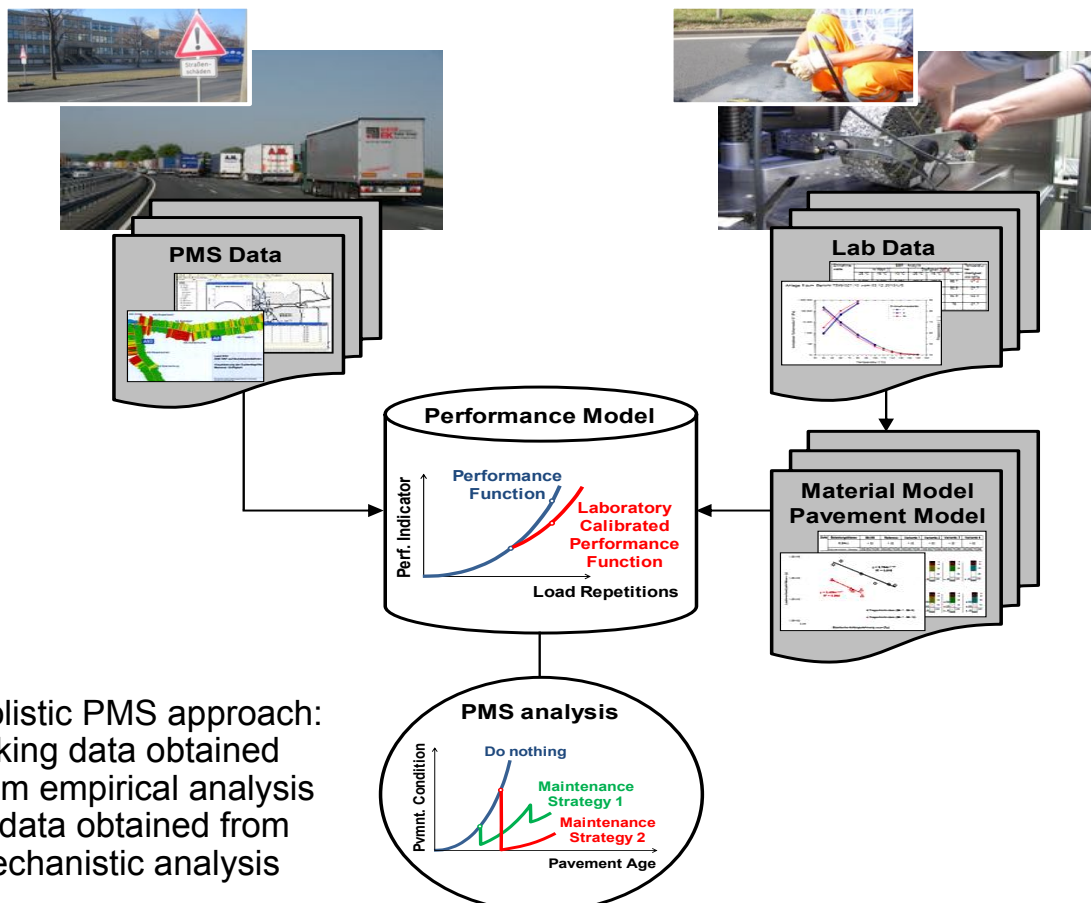
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# Linking empirical with mechanical information

- use information obtained from material testing in the laboratory and from structural performance modeling
- calibration of empirical performance function



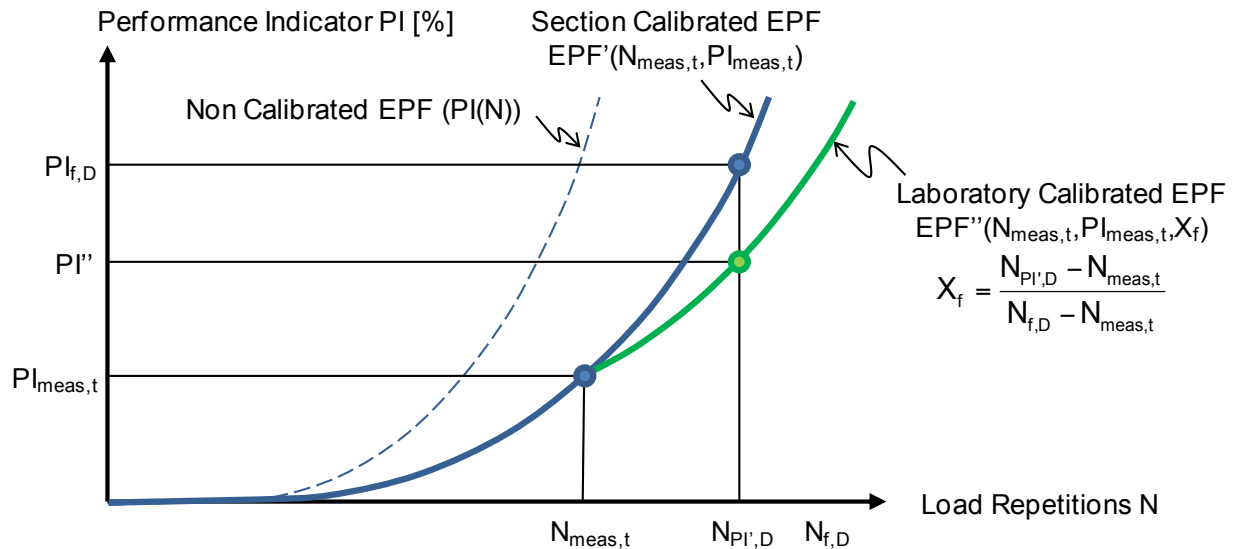
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Holistic PMS approach:  
linking data obtained  
from empirical analysis  
to data obtained from  
mechanistic analysis

## Linking empirical with mechanical performance

- ERAnet-Road-Project “InteMat4PMS” (2012)
- PROMAT project (2016)



$$PI''_{t+N} = PI_{meas,t} + EPF''(\Delta N) = PI_{meas,t} + X_f \cdot EPF'(\Delta N) \quad \text{where } \Delta N = N_t - N_{meas,t}$$

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## Predicting asphalt pavement performance

### Concluding remarks

- need for consideration of thermo-mechanical behavior of bituminous mixtures and pavement structures
- the influence of load *and* temperature on damage evolution is a key issue
- for life-cycle-analysis of pavement structures linking empirical *and* mechanical information is of advantage

**Thank you for your contribution!**